



**GRAND RENEWABLE ENERGY PARK
STORMWATER MANAGEMENT REPORT**

DRAFT

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1.0 Introduction

Stantec Consulting Ltd. (Stantec) has been retained to complete the design of the stormwater management (SWM) works associated with two components of the Grand Renewable Energy Park, namely the substation and operation and maintenance facilities within the solar project. The facilities are generally located east of Mount Olivet Road / Bains Road and north and south of Haldimand Road 20, respectively, as shown on Figure 1.

This report documents the SWM strategy and designs developed to mitigate against potential off-site water quality and quantity impacts associated with the development of the subject facilities, both during- and after-construction.

The SWM design approach involved the following study components:

- Complete hydrologic models for the existing and proposed conditions to determine potential negative impacts of project development, if left uncontrolled
- Finalize the SWM designs to control the site runoff in a manner consistent with current requirements (water quality and water quantity)
- Define an erosion and sediment control plan to minimize the potential for erosion and off-site transfer of sediment during construction
- Define a post-development monitoring program to confirm that implemented measures achieve the design level of treatment / control
- Outline an anticipated operation and maintenance program to be implemented by the Owners
- Summarize the study by identifying conclusions and recommendations

The primary guidance document referenced in the completion of the proposed SWM design is the Stormwater Management Planning and Design (SWMPD) Manual, Ministry of the Environment, 2003. The Natural Heritage Assessment Report (Stantec, January 2011) documented site-specific background information relating, in particular, to the characteristics of the receiving systems that was utilized in the establishment of appropriate SWM control criteria and considered in the siting of associated infrastructure.

2.0 Existing Conditions

The existing conditions of the substation and operations and maintenance (O&M) sites are similar, given the geographic proximity. As delineated on Figures 2.0 and 6.0, the drainage catchments for both facilities are relatively small and comprise primarily agricultural land use under existing conditions. The total catchment areas analyzed for the purposes of comparing with proposed development runoff conditions include 34.1 ha and 24.9 ha for the substation and O&M facilities, respectively.

The existing topography of the site is relatively flat, with slopes in the range of 0.5%, and the majority of at-surface soils comprised of a combination of Haldimand and Lincoln clays, as identified on mapping OMAFRA / MNR. Runoff curve numbers, as defined by the US Soil Conservation Service (SCS) and other hydrologic parameters were calculated for each existing catchment based on land use and soil type and are provided in Appendices B and C.

Drainage from both sites is conveyed to a tributary of Wardells Creek, though the O&M facility is more immediately adjacent to the receiver, whereas drainage from the substation facility area will be conveyed along roadside ditches and/or intermittent tributaries prior to its confluence with the more significant tributary system.

The locations of the substation and O&M facilities and/or their associated access roads receive drainage from upstream, external contributing lands that is proposed for diversion around the infrastructure area using grassed swales similar in characteristic to the existing drainage systems. Diversion of such flows minimizes the potential for unnecessary mixing of runoff from the subject sites and/or compromising the effectiveness of the proposed treatment systems. Additional discussion in this regard is provided within Section 4.

3.0 Stormwater Management Objectives / Criteria

The primary stormwater management philosophies, or fundamental principles, developed within the SWMP generally mimic those promoted by the SWMP Planning and Design Manual and reflect current standards of practice. The general approach incorporates a Best Management Practice (BMP), multi-component 'treatment train' approach and includes emphasis on at-source, conveyance, and end-of-pipe treatment controls. The following general objectives guided the development of the site SWM designs:

- • Minimize the impact of post development conditions on downstream areas
- • Maintain, protect, and enhance the existing watercourse function
- • Maintain and preserve the quality of runoff discharge to the receiving waters

Lacking a formally established subwatershed management plan, the SWM criteria were based on an assessment of the characteristics of the receiving systems to define appropriate level of quality control, as well as typical standards of drainage and riparian rights law to define the approach to water quantity control. In this regard, the criteria proposed for implementation include:

- • Provide peak flow control of runoff for the return-period events to ensure that pre-development release rates are achieved or reduced to minimize the potential for peak flow related impacts on downstream landowners
- • Provide an Enhanced (formerly Level 1) degree of water quality control that aims to reduce effluent total suspended solids (TSS) concentrations by 80% through use of engineered treatment systems. While the reduction of TSS loads is also considered to achieve corresponding reductions in other contaminants, given their adsorption to the suspended particulates, consideration of spills potential at the subject facilities should also be incorporated to minimize the potential off-site of oils, greases, etc.

4.0 Proposed Conditions

The relative size and impervious coverage characteristics of the proposed collector substation and operations and maintenance (O&M) facilities, as well as their associated access roads, are such that negative impacts on stormwater quality and quantity could occur if not mitigated through the use of SWM detention/treatment systems.

4.1 SUBSTATION

The collector substation will be built to accumulate the power circuits from the wind and solar generation equipment. The substation will be located near Haldimand Road 20 and Mt. Olivet Road within the solar lands of the Project Location.

The collector substation will consist of a gravel pad area containing associated infrastructure that is 85 m by 85 m in size with an approximately 1 km long access road entering from Haldimand Rd 20. The gravel surface, traveled portion of the access road is approximately 8 m wide and is of rural cross-section, with grassed swale drainage ditches on either side. For the purposes of proposed conditions hydrologic modeling, the gravel surfaces of substation pad and access road are considered to be 100% impervious.

The delineation of the proposed drainage catchments is provided on Figure 3.0 and is summarized as follows:

- Catchment 201 – 10.62 ha of solar module fields draining to a proposed diversion swale immediately east of the access road discharging to the existing drainage ditch along Haldimand Road 20
- Catchment 202 – 18.25 ha of predominantly agricultural land (1 solar module) draining to a proposed drainage swale on the west side of the access road discharging to the existing drainage ditch along Haldimand Road 20
- Catchment 203 – 2.53 ha of the proposed substation block, proposed access road right-of-way (including grassed swale drainage ditches), and the 0.5 ha SWM block. All major and minor flows are to be conveyed to the proposed SWM facility
- Catchment 204 – 3.20 ha of external lands which are to be diverted along the east side of the proposed SWM facility to the existing drainage ditch located along Haldimand Road 20
- SCS curve numbers (CNs) and hydrologic parameters were calculated for each catchment based on land use and soil type and are provided in Appendix B.

4.2 OPERATIONS & MAINTENANCE FACILITY PROPOSED CONDITIONS

The operations and maintenance facility is proposed on the south side of Haldimand Road 20 opposite the solar farm land area, just east of Mount Olivet Road / Bains Road. The building will be a prefabricated engineered structure measuring 46 m wide by 85 m long by 7 m high. It will be founded on concrete foundations that are extended below grade to below the frost line. The access road and equipment/material storage area surrounding the buildings are to be gravel-surfaced and assumed, for the purposes of hydrologic modeling herein, to be 100% impervious coverage.

A 10 m wide gravel-surfaced access road servicing the O&M facility will intersect with Haldimand Road 20 northeast of the facility, just east of a woodlot, and proceed due south and west to the building parking area. Grassed swale drainage ditches on either side will convey road runoff to the SWM facility.

The delineation of the proposed conditions drainage catchments is provided on Figure 7.0 and is summarized as follows:

- Catchment 201a – 10.9 ha of solar module field / agricultural area east of the site draining to a grassed diversion swale immediately east of the access road, discharging to the existing receiver south of the O&M facility
- Catchment 201b – 0.2 ha catchment containing the northerly 1/3 of access road (~100 m) and associated grassed swale drainage ditches
- Catchment 202a – 6.3 ha of solar module field / agricultural area east of the site draining to a grassed diversion swale immediately east of the access road, discharging to the existing receiver south of the O&M facility
- Catchment 202b - 5.8 ha containing the proposed O&M facility, the southerly 2/3 of proposed access road (including grassed swale drainage ditches), and the SWM block. All major and minor flows are to be conveyed to the proposed SWM facility
- Catchment 202c – 1.7 ha of solar module field east of the site draining to a grassed diversion swale immediately east of the O&M facility and associated SWM, discharging to the existing receiver south of the O&M facility

SCS curve numbers (CNs) and hydrologic parameters were calculated for each catchment based on land use and soil type and are provided in Appendix C.

5.0 Proposed Stormwater Management System

The proposed SWM systems include a ‘treatment train’ approach to mitigate potential impacts from the proposed land use changes in accordance with the MOE recommended hierarchy of SWM approaches, ranging in preference from lot level controls, to conveyance controls, to end-of-pipe controls. The SWM designs for both the substation and operations and maintenance facilities utilize a rural cross-section for the access roads, complete with shallow-slope grassed swale drainage ditches, and end-of-pipe detention basins. Details of all aspects of the proposed SWM systems are outlined in the following sections, with additional design information and modeling included within the Appendices. Summary tables outlining the most important design and operating characteristics for each of the SWM Facilities are provided within the following sections.

5.1 HYDROLOGIC MODELING

Hydrologic models were prepared to simulate proposed drainage conditions for the subject developments using the Stormwater Management Hydrologic Model (SWMHYMO) software. The models predicted flows for the existing and proposed development conditions and assessed the design of SWM systems within the collector substation and O&M Management Facility to ensure that the previously discussed stormwater criteria are achieved.

To assess the proposed SWM designs ability to mimic existing conditions hydrology, the

2-100 year, 24-hour SCS storms derived using the Simcoe Station IDF parameters were analyzed. The parameters used to define the design storm events are summarized on Table 5.1, below:

Table 5.1: Rainfall Events – 24-hour SCS Storm Event Depths (Simcoe Station)

Storm	Depth (mm)
1:2-year	50.7
1:5-year	66.8
1:10-year	77.4
1:25-year	90.8
1:50-year	100.7
1:100-year	110.6

Schematics of the SWMHYMO models, as well as input and output files, are appended.

5.2 SUBSTATION FACILITY

Drainage generated within this site is conveyed along approximately 1000 m of shallow-slope (~0.5%) grassed swale drainage ditches and temporarily detained within a dry end-of-pipe SWM facility before discharging to a road drainage ditch along Haldimand Road 20. Contributory drainage generated with the catchments upstream of and surrounding of the proposed substation and SWM facility will bypass the developing portions of the site as well as the SWM conveyance / treatment systems via vegetated diversion swales. All discharge (diverted or treated) is to the north ditch of Haldimand Road 20, in a manner consistent with that which occurs under existing conditions. Drainage from this system ultimately contributes to a tributary of Wardells Creek.

An impervious coverage value of 100% has been assumed for the substation pad and the 8 m wide access roads; the grassed swale drainage ditches represent an additional 2.5 m wide pervious zone paralleling either side of the access road along its entirety. Given the typically 'dry' character of the SWM facility, this area (0.5 ha) is also considered to be pervious for the purposes of the hydrologic assessment.

Additional details regarding the drainage area characteristics and SWM designs for the substation facility are provided in Appendix B.

5.2.1 Water Quality Control

The drainage outlet for the substation facility and access road is a roadside ditch along Haldimand Road 20, which eventually discharges to a tributary of Wardells Creek. In recognition of the wetland associated with the tributary downstream (near the O&M facility discharge location), the proposed grassed swale drainage ditch / dry end-of-pipe detention facility treatment train has been designed to achieve an Enhanced (formerly Level 1) degree of water quality protection, as identified within Section 3.

Scientific literature documenting the pollutant removal capabilities of grassed swales indicates widely variable results, dependent to a large extent on the design characteristics of the systems. Critical design aspects to be considered in the improvement of performance include the maximization of bottom width and swale length, and minimization of longitudinal slope, all of which improve runoff / vegetation contact and minimize flow velocities. These aspects have been incorporated into the subject design through incorporation of very long flow length (~1000 m), broad trapezoidal cross-section relative to the predicted flow rates, and shallow longitudinal slope (~0.5%) mimicking the existing topography. It can reasonably be expected that the swales will provide at least 60-70% TSS reduction.

As per the MOE design guidelines, the TSS reduction capabilities of dry, end-of-pipe SWM facilities are generally limited to approximately 60%, primarily achieved through the temporary detention of flows giving sediments an opportunity to settle out and be captured. Performance

in this regard has been maximized within the subject facility design through the use of the smallest permitted quality control orifice (50 mm diameter), maximizing the detention time.

Extended detention drawdown control has been incorporated within the design to achieve the 24-hour drawdown of the 213 m³/ha volume specified by the MOE SWMPD Manual. In order to meet these criteria with such a limited contributing drainage area, the low-flow quality control orifice has been sized at 50 mm diameter (minimum permitted), providing an approximate 29-hour drawdown of the 433 m³ water quality volume.

The vegetated characteristic of dry facilities also provides sediment removal and nutrient uptake benefits. A planting strategy utilizing careful selection of plant species tolerant of a range of moisture conditions, and their strategic location in and around the basin will stabilize banks, mitigate temperature increases, deter waterfowl from nesting within the area, improve performance, and provide aesthetic and safety benefits.

Table 5.2: Substation Facility SWM System Water Quality Design Characteristics

General Parameters	Basin Value
Total Contributing Area to SWM Facility (ha)	2.53
Total Area to SWM Facility requiring quality control (ha) ¹	2.03
Imperviousness of Contributing Area (%)	60
Total Area of SWM Block (ha)	0.50
SWM Basin Water Quality Parameters	
Water Quality Unit Volume Requirements as per SWMPD Manual (m ³ /ha)	213
Total Required Water Quality / Extended Detention Volume (m ³)	433
Extended Detention Volume Provided (m ³)	509
Peak Release Rate for MOE Extended Detention (m ³ /s)	0.044
Extended Detention Drawdown Time Required (hrs)	24
Extended Detention Drawdown Time Provided (hrs)	30

Notes:

1 Drainage Area for Quality control represents total drainage area to SWMF less the area of the SWM Block itself.

5.2.2 Water Quantity Control

As outlined in Section 3.0, water quantity controls within the proposed development are required to minimize the potential for peak flow related impacts on receiving systems and downstream landowners. A dual-stage outlet structure is proposed for implementation at the dry SWM facility, as detailed on Figure 5.0. In addition to the water quality control described above, this system includes a second orifice, also contained within the perforated CSP riser structure. A detailed stage-storage-discharge analysis for the facility is provided within Appendix B. Target peak flow values and proposed post-development controlled values for the 2-100 year return-period rainfall events are provided in Table 5.3.

In the event of an extreme rainfall event (>1:100-year return-period) or blockage of the SWM facility outlet, flows would spill southerly to the Haldimand Road 20 ditch via a proposed rip-rap lined emergency overflow weir.

Table 5.3: Substation Facility SWM Quantity Control Design Characteristics

	Return-Period Event (yrs)				
	2	5	10	25	100
Existing Conditions Peak Discharge (m ³ /s) ¹	0.61	0.94	1.17	1.47	1.93
Proposed Conditions Peak Discharge (m ³ /s) ¹	0.57	0.88	1.10	1.38	1.80
Proposed SWM Facility Discharge (m ³ /s)	0.04	0.07	0.09	0.11	0.12
Maximum Ponding Elevation (m)	199.85	199.85	199.94	199.98	200.04
Maximum Storage Volume Used (m ³)	500	650	760	900	1,130

Notes:

1 The comparison point of existing and proposed conditions peak discharge values is at the Haldimand Road 20 ditch and includes all areas for which runoff is being detained, treated, or diverted, as illustrated on Figures 2 and 3.

5.2.3 Stormwater Management Facility Design

The design of the end-of-pipe SWM facility adheres to standard principles and characteristics recommended or required by the SWMP Design Manual (MOE, 2003). The following list of design characteristics, read in conjunction with Figures 4.0 and 5.0, outlines all significant design aspects and rationales:

- The SWM basin has been designed as a dry, extended detention facility with sufficient active storage volumes to achieve a Basic (formerly Level 3) degree of protection
- Water quantity controls will be implemented to limit peak discharge rates for the return-period rainfall events (2-100 year) to existing conditions rates, as determined within the current work
- Given the proximity of access for maintenance around the entire facility, the shallow depth to pond invert, and the 'dry' nature of the pond bottom, generally making maintenance tasks a simpler procedure than with a 'wet' facility, an access road has not been incorporated into the design
- Internal side slopes of 3:1 are proposed as the safety concerns normally associated with a 'dry' facility are not a concern given the shallow, 'dry' nature of the proposed design
- The design of the required outlet structures incorporates a dual-stage configuration for control across the range of rainfall-runoff events, as outlined on Tables 5.3 and 5.4

- Operation, maintenance, and monitoring (OMM) of the stormwater management facility will be the responsibility of the Owners. Additional detailed discussion pertaining to the anticipated OMM program is provided within Section 8 of this report

Table 5.4: Substation Facility SWM Pond Outlet Structure Characteristics

Outlet Structure Parameter	
Water Quality Orifice Plate Diameter (within CSP Riser)(mm)	50
Extended Detention Orifice Plate Invert (m)	199.45
Quantity Control Orifice Diameter (within CSP Riser)(mm)	350
Quantity Control Orifice Invert (m)	199.65
Trapezoidal Overflow Spillway Elevation (m)	200.10
Trapezoidal Overflow Spillway Bottom Width (m)	3
Trapezoidal Overflow Spillway Side Slopes	3

5.3 O&M STORMWATER MANAGEMENT FACILITY

Drainage generated within this site is conveyed via shallow-sloped (~0.5%) grassed swale drainage ditches and temporarily detained within a constructed wetland end-of-pipe SWM facility. Contributory drainage generated with the catchments upstream of and surrounding of the proposed O&M facility and SWM facility will bypass the developing portions of the site as well as the SWM conveyance / treatment systems via vegetated diversion swales. All discharge (diverted or treated) is to an existing drainage draw that conveys flows to a tributary of Wardells Creek, in a manner consistent with that which occurs under existing conditions.

An impervious coverage value of 100% has been assumed for the O&M building/parking area and the 10 m wide access road; the grassed swale drainage ditches represent an additional 5 m wide pervious zone paralleling either side of the access road along its entirety. Given the permanent pool proposed within the wetland SWM facility, this block is assumed to be 50% 'impervious' for the purpose of assessing hydrologic characteristics.

Additional details regarding the drainage area characteristics and SWM designs for the O&M facility are provided in Appendix C.

5.3.1 Water Quality Control

The O&M SWM facility has been designed as a constructed wetland with an average permanent pool depth of 0.3 m within the wetland component of the facility. Constructed wetlands offer the dual benefits of dilution and settling of sediment within the forebay and the wetland components of the facility, with the added benefit of biological removal of pollutants (i.e., nutrient uptake) via the wetland plantings. Careful selection of plant species and their location in and around each basin helps stabilize banks, mitigate temperature increases, deter waterfowl from nesting within the area, improve performance, and provide aesthetic and safety benefits.

A sediment forebay has been provided at the inlet to the O&M facility and is designed with a maximum depth of 1.5 m. The design depth is that anticipated immediately following construction and after sediment clean-out operations. The minimum design operating depth of 1.0 m is that which follows a period of sediment collection to 0.5 m depth, and coincides with the point immediately prior to the required clean-out operations. Maintenance of at least 1.0 m of permanent pool in the forebay at all points in the sediment accumulation / clean-out cycle minimizes the potential for scour and re-suspension of previously settled sediments.

It is noted that the proposed forebay area exceeds the MOE recommended design criterion of $\leq 20\%$ of the total permanent pool surface area. The primary rationale for that design criterion, however, reflects the fact that the volumetric sizing criteria for constructed wetland-type facilities relies on the wetland vegetation component of the facility to provide the majority of the water quality treatment functions. In other words, it is contrary to the design sizing guidance to utilize deep water areas, such as those in forebays, to achieve the volumetric storage requirements established as appropriate for constructed wetland facilities. Within the current design, the permanent pool volumetric sizing requirements, as defined by the SWM Planning and Design Manual (MOE, 2003), are achieved within the wetland component of the facility without accounting for the storage volume provided within the forebay. It is concluded, therefore, that the proposed facilities achieve the target of the MOE in this regard.

Specifics of the SWM facility design characteristics pertaining to the provision of water quality treatment are summarized on Table 5.5.

Table 5.5: O&M SWM Facility Water Quality Design Characteristics

General Parameters	
Total Contributing Area to SWM Facility (ha)	6.0
Imperviousness of Contributing Area (%)	50%
Surface area of Permanent Pool (ha)	2,813
SWM Basin Water Quality Parameters	
Forebay Invert Elevation (m)	195.80
Water Quality Volume Requirements as per SWMPD Manual (m ³ /ha)	99
Total Required Water Quality Volume (m ³)	594
Required Extended Detention Volume (m ³)	240
Extended Detention Volume Provided (m ³) ²	289
Required Permanent Pool Volume (m ³)	353
Permanent Pool Volume Provided (Total – above sediment storage) (m ³) ²	1,078
Permanent Pool Volume Provided within wetland component alone (m ³) ²	546
Peak Release Rate for Extended Detention (Quality Control) (m ³ /s)	0.003
Extended Detention Drawdown Time (Quality Control) (hrs)	27
Forebay Parameters	
Required Forebay Length (m)	6
Actual Forebay Length (m)	30
Sediment Storage Volume Provided (m ³)	90
Clean-Out Frequency (yrs)	12

5.3.2 Water Quantity Control

As outlined in Section 3.0, water quantity controls within the proposed development are required to minimize the potential for peak flow related impacts on receiving systems and downstream landowners. A dual-stage outlet structure is proposed for implementation at the SWM facility, as detailed on Figure 9.0. In addition to the water quality control described above, this system includes a second orifice, also contained within the perforated CSP riser structure. A detailed stage-storage-discharge analysis for the facility is provided within Appendix C. Target peak flow values and proposed post-development controlled values for the 2-100 year return-period rainfall events are provided in Table 5.6.

In the event of an extreme rainfall event (>1:100-year return-period) or blockage of the SWM facility outlet, flows would spill to the receiving drainage system via a proposed rip-rap lined emergency overflow weir.

Table 5.6: O&M SWM Facility Quantity Control Design Characteristics

	Return-Period Event (yrs)				
	2	5	10	25	100
Existing Conditions Peak Discharge (m ³ /s) ¹	0.54	0.84	1.04	1.32	1.74
Proposed Conditions Peak Discharge (m ³ /s) ¹	0.44	0.70	0.88	1.11	1.46
Proposed SWM Facility Discharge (m ³ /s)	0.05	0.09	0.11	0.12	0.15
Maximum Ponding Elevation (m)	197.71	197.83	197.91	198.02	198.18
Maximum Storage Volume Used (m ³)	1,290	1,720	2,030	2,450	3,100

5.3.3 5.3.3 Stormwater Management Facility Design

The design of the end-of-pipe SWM facility adheres to standard principles and characteristics recommended or required by the SWMP Design Manual (MOE, 2003). The following list of design characteristics, read in conjunction with Figures 8.0 and 9.0, outlines all significant design aspects and rationales:

- The facility has been designed as a constructed wetland with sufficient permanent and active storage volumes to achieve an Enhanced (formerly Level 1) degree of protection, in recognition of the Provincially Significant Wetland (assumed) present within the valley of the receiving watercourse
- Water quantity controls have been incorporated to provide extended detention for erosion control and to limit peak discharge rates for the return-period rainfall events (2- to 100-year) to existing conditions flow values
- Anticipated clean-out frequencies for the facility’s forebay, as summarized in Table 5.5, is 12 years
- The design of the outlet structure incorporates a dual-stage configuration for flow control across the range of rainfall-runoff events, as outlined on Table 5.7
- Design permanent water depths within the forebays are set at 1.5 m and are anticipated to be present immediately after construction and following future sediment clean-out procedures. The provision of 1.5 m depth within the forebay allows for 0.5 m sediment accumulation prior to recommended clean-out while maintaining 1.0 m of permanent pool storage, thereby minimizing the risk of scour and re-suspension of previously settled sediments throughout the deposition / clean-out cycle
- Operation, maintenance, and monitoring (OMM) of the stormwater management facilities will be the responsibility of the Owners. Additional detailed discussion pertaining to the anticipated OMM program is provided within Section 8.0 of this report.

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Table 5.7: O&M SWM Facility Outlet Structure Characteristics

Outlet Structure Parameter	
Water Quality Orifice Plate # 1 Diameter (within CSP Riser)(mm)	75
Water Quality Orifice Plate # 1 Invert (m)	197.30
Quantity Control Orifice Plate # 2 Diameter (within CSP Riser)(mm)	300
Quantity Control Orifice Plate # 2 Invert (m)	197.50
Trapezoidal Overflow Spillway Elevation (m)	198.20
Trapezoidal Overflow Spillway Bottom Width (m)	5
Trapezoidal Overflow Spillway Side Slopes	5

6.0 Erosion and Sediment Control

In order to control erosion and transportation of sediment off-site, an Erosion and Sediment Control Plan (ESC) has been developed and will be implemented during the construction process. The plan focuses on the protection of downstream receivers, namely the adjacent wetland units and receiving watercourses.

The Greater Golden Horseshoe Conservation Authorities Erosion and Sediment Control Guideline for Urban Construction (2006) document was used to determine the erosion potential of the site. Those factors affecting the erosion potential of a given site considered in the assessment include slope gradient, slope length, and soil texture. The relative magnitude of erosion potential guides the development of an appropriate erosion control strategy.

Existing conditions gradients on the subject lands can be summarized as generally gentle

(< 2%) with predominantly long slope lengths (greater than 30 m). Site soils are comprised primarily of Haldimand and Lincoln clays, which are considered to represent a low erodibility potential. Finally, the potential for negative impact on the receiving systems should sediment be transported off-site during construction, should be considered. In the case of the subject lands, the potential for impact on the receiving natural systems is high. In consideration of all of the evaluation parameters described above, the overall erosion potential for the site is considered to be “moderate”.

Most of the various construction activities will result in the disturbance of at-surface soils to various extents, ranging from construction traffic to topsoil stripping and/or grading activities involving cutting or filling, all of which expose the underlying earth to potential erosion and sediment transport to off-site locations. In all instances where the potential for erosion is identified a series of control measures will be implemented including, but not limited to:

- Erect silt fence before grading begins on the downstream side of the area to be graded to protect the downstream lands from potential sediment transport that may be entrained in overland flows.
- Provide a construction entrance feature (“mud mat”) at all site entrances to minimize the transport of sediment on construction vehicle tires
- Direct runoff via swales and erosion control berms (where necessary) to sediment control measures to ensure that no untreated runoff is discharged from the site.
- Utilize the proposed end-of-pipe SWM facilities as temporary sediment control measures.

- Install temporary rock check dams in swales where appropriate to help attenuate flows, reduce erosive velocities, and encourage sediment deposition
- Immediately stabilize all disturbed areas not subject to construction activities within 30 days, according to OPSS 572.

In order to ensure the effectiveness of the various erosion and sediment control measures, an appropriate inspection and maintenance program is necessary. The inspection activities will include:

- Inspection of the erosion and sediment controls after each significant rainfall event or weekly, whichever is more frequent
- Inspections should include all silt fence installations, rock-check dams, the sediment control facility, outlets and vegetation
- Submission of regular monitoring results to the LPRCA during active construction periods

7.0 Monitoring Program

The proposed monitoring program includes detailed monitoring requirements for the during-construction and post-construction stages of development. Within each stage of the program, monitoring requirements with respect to water quality and quantity have been detailed.

7.1 DURING-CONSTRUCTION MONITORING

Grading and servicing activities constitute the during-construction monitoring stage within which the minimization of potential stormwater impacts is primarily concerned with the control of erosion and off-site transport of sediment.

In addition to the erosion and sediment control inspections discussed in Section 6, the following elements will be monitored and documented during the construction period:

- i. The general condition of the discharge point from the SWM facilities
- ii. The stability of the SWM facilities embankment slopes and the condition of plantings
- iii. The performance and sedimentation levels within the SWM facilities

An annual report will be submitted to the LPRCA summarizing the monitoring results and will make appropriate recommendations for future monitoring if necessary.

7.2 POST-CONSTRUCTION MONITORING

Implementation of a post-construction monitoring program provides the data necessary to assess that the SWM system is functioning as designed and achieves the target control values for treatment. Within the current study, the post-construction period is defined as the two-year period after construction and stabilization of the associated infrastructure (substation, O&M facility, and associated access roads).

The monitoring of SWM facility performance will represent a key component of the post-construction period program, with an objective of confirming the operational characteristics predicted as part of the design process, identifying any discrepancies, and implementing remediative approaches in the event that such are required. Operational inspections will focus on the surface water quantity and quality characteristics of the facilities. Monitoring of inflow and outflow conditions during and immediately after rainfall events, combined with observations of water level fluctuations, will confirm hydrologic characteristics of the contributing catchment area and SWM facility response to the associated runoff. Laboratory testing of grab samples obtained at the inlet and outlet of the SWM facilities on a quarterly basis will confirm pollutant removal characteristics. Water quality parameters to be tested include TSS, TP, DO and E.coli.

Concurrent with the grab sampling / laboratory analysis testing program, in situ air and water temperatures will also be recorded.

Further to the operational characteristics, inspections and monitoring of the general condition of stormwater management infrastructure is discussed in detail in the following section, dealing with operations and maintenance.

Annual reports will be submitted to the LPRCA summarizing the monitoring results and will make appropriate recommendations for future monitoring if necessary.

8.0 Operations and Maintenance Program

The ability of any SWM practice to continue functioning as designed relies on the development and implementation of an operations and maintenance program. While the following sections outline the details of the program components anticipated at the time of design, the adoption of a broader adaptive management philosophy acknowledges the potential for refinement of the program to reflect actual field observations recorded as part of the monitoring program, described in the previous section.

The various components of the stormwater systems proposed for implementation within the Grand Renewable Energy Park are typical of standard practice and represent straightforward activities. Typical SWM measures incorporated within the proposed strategy include the use of grassed swale (ditch) conveyance systems as well as dry and constructed wetland end-of-pipe quality / quantity control facilities.

8.1 GRASSED SWALE (DITCH) CONVEYANCE SYSTEMS

Grassed swale (ditch) conveyance systems represent a familiar, passive, and simple type of stormwater management practice, with operational and maintenance requirements to match. Generally speaking, the treatment benefits of a grassed swale are the result of the contact between the flows being conveyed and the vegetation within the swale. Given this, inspection, operational, and maintenance activities can be generally limited to:

- Routine observations as to the presence of trash/debris within the swale that could be conveyed downstream and/or affect the conveyance capacity of the system and removal of same as needed.
- For the first two years following construction, a semi-annual walking inspection should be completed to identify areas of bare soil and/or the formation of erosive gullies (annually thereafter). Remediative efforts would typically involve re-grading the area and/or re-vegetating with sod or appropriate seed mix, with fertilizer and water applied as necessary to ensure germination and stabilization.
- Concurrent with the walking inspections, a visual assessment of any areas of isolated ponding or sediment build-up should be identified. Minor areas of ponding can be resolved with re-grading / re-stabilization, if the magnitude of associated nuisance warrants such action. From a stormwater management perspective, there are no functional concerns associated with ponding and, therefore, remediation is not strictly required. Excessive sedimentation is an issue requiring attention if it remains in a non-vegetated condition and is, therefore, prone to re-suspension and transport downstream, if it creates an isolated ponding area as described above, or if it occurs to an extent that it impacts on the conveyance capacity of the swale. If any such condition occurs, the sediment should be removed and the area re-stabilized.

- Vegetation management is not a strict requirement in that excess growth will serve to improve water quality treatment benefits. If the density of vegetation reaches a level where conveyance capacity is impacted, a cutting operation should be undertaken. A minimum vegetation height of 0.15 m (6") should be maintained.

8.2 END-OF-PIPE STORMWATER MANAGEMENT FACILITIES

Long-term operation and maintenance responsibilities at end-of-pipe SWM facilities include regular facility inspections and the implementation of associated remediative actions. For the first two years following construction, inspections should be undertaken following each significant rainfall event (averaging approximately 4 inspections / year) to gain confidence that the facilities are functioning as designed. Following this period, the frequency of inspections can be reduced to an annual or as-needed basis.

The types of information that operations staff should be recording and rectifying, if required, include questions such as:

- Are the regular pond levels above the permanent pool elevation after the predicted extended detention drawdown times outlined herein? This situation could be indicative of outlet blockage by trash or sediment; visual inspection should be completed to confirm.
- Within a 'wet' SWM facility, such as the constructed wetland SWM facility proposed as part of the O&M infrastructure design, pond levels should be assessed to determine if they are lower than the normal permanent pool elevation. Such a condition could be indicative of a blockage of the inlet or leakage through the pond's invert; visual inspection of inlet should be completed to confirm clear passage. Given the predominantly clay characteristic of on-site soils at location of the subject facility, significant leakage is not anticipated. Weather conditions in the days and weeks leading up to the inspection should also be considered as evaporative losses during a hot, dry spell could be significant.
- Is there damage to facility structures including headwalls, pipes, berms, maintenance accesses, etc.? Maintenance requirements in this regard should be performed on an as required basis.
- What are the visual characteristics of water in the facilities (i.e., oily sheen, frothy, colour, etc.)? Issues in this regard could be indicative of an upstream spill and the need for cleanup.
- Is the vegetation around the facilities unhealthy or dying? Are there areas around the ponds with easy access to open water? Deficiencies in this regard could be indicative of either poor species selection at design, or any number of chronic causes. Lack of vegetation, particularly around the water's edge, increase attractiveness and use by

waterfowl, often leading to degradation in effluent water quality (i.e., increased bacteria loadings). Replanting should be undertaken to ensure sufficient vegetation densities.

- Sediment depth and oil accumulation within the forebay or main cell. Within a 'wet' facility, sediment depth can be measured with a graduated pole at a standardized location (can be identified with a marker that is left in the facilities). Sediment should be removed when the permanent pool depth is reduced to 1.0 m within the forebay areas. Owing to the increased sediment loadings anticipated during construction, the clean-out frequencies estimated during the design process might be reduced during the interval prior to complete stabilization of the upstream contributing drainage areas. In any event, the removal and disposal of sediment from all facilities should be completed by a qualified party and/or licensed contractor.
- Erosion around outlet structures or downstream areas requiring stabilization work. All noticeable erosion and damage within and immediately outside the basin should be repaired and stabilized as quickly as possible.
- Draining of the O&M SWM facility will be accomplished through pumping when maintenance is required.

9.0 Conclusions

Based on the preceding report, it is concluded that the proposed SWM strategies for the substation and operations and maintenance facility sites are appropriate for the provision of required water quality and quantity control. Stormwater management approaches including diversion swales designed to route flows from external drainage areas around the proposed facilities, grassed swale drainage ditch conveyance systems, and end-of-pipe stormwater management basins (dry and constructed wetland type for the substation and O&M facilities, respectively) provide:

- Enhanced (formerly Level 1) degree of water quality protection
- Water quantity controls to ensure that discharge to the proposed conditions peak flow discharge rates to the receiving systems are at or below those predicted for the same rainfall events under existing conditions

Site topography, native soils, slope lengths, and the relative sensitivity of adjacent areas and downstream receivers are such that the site is considered to represent a 'moderate' erosion potential. In recognition of such conditions, an ESC strategy that utilizes a multi-component approach aimed at minimizing erosion potential across the graded site and providing a series of sediment control measures to maximize on-site capture of any eroded material has been developed.

During-Construction and Post-Construction Monitoring Programs will be implemented to record data to ensure that they are functioning as designed and achieving the required levels of control. Following an adaptive management approach, the monitoring work programs will continue to evolve, as necessary, in consultation with the respective agencies (LPRCA, MOE), as information is compiled.

STANTEC CONSULTING LTD.

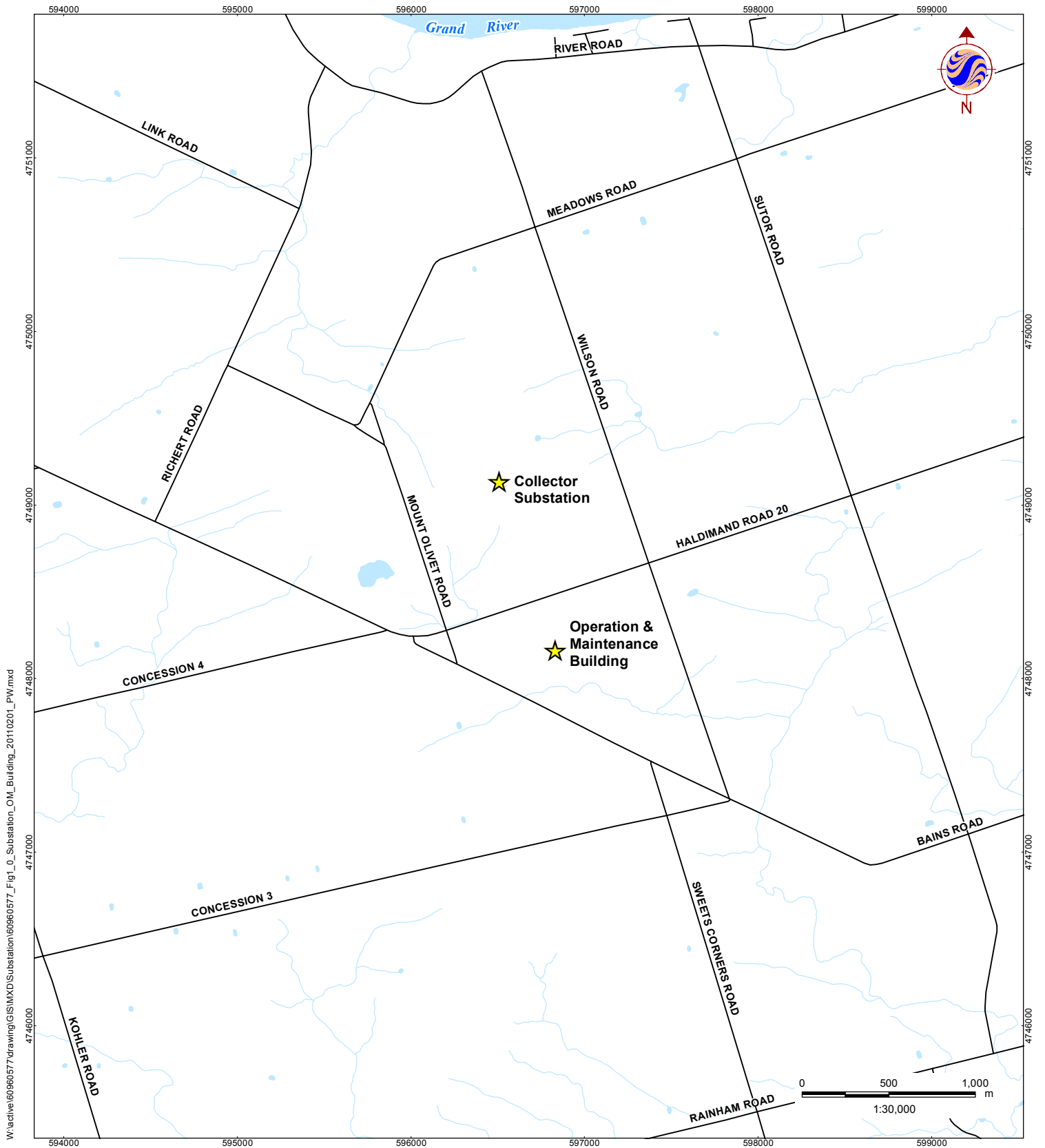
DRAFT

Scott Robertson, P.Eng.

Associate, Water Resources Project Manager

Appendix A

Figures



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February 2011
160960577

Legend

- Electrical Transmission Component
- Road
- Watercourse
- Waterbody

Notes

1. Coordinate System: UTM NAD 83 - Zone 17 (N).
2. Base features produced under license with the Ontario Ministry of Natural Resources © Queens Printer Ontario, 2011. © Samsung, 2011.
3. Image Source: © Terrapoint, 2011 - Imagery Date: July 2009; Grand River Conservation Authority © First Base Solutions, 2011 - Imagery Date: Spring 2006.

Client/Project

**SAMSUNG, PATTERN & KEPCO (SPK)
GRAND RENEWABLE ENERGY PARK**

Figure No.

1.0

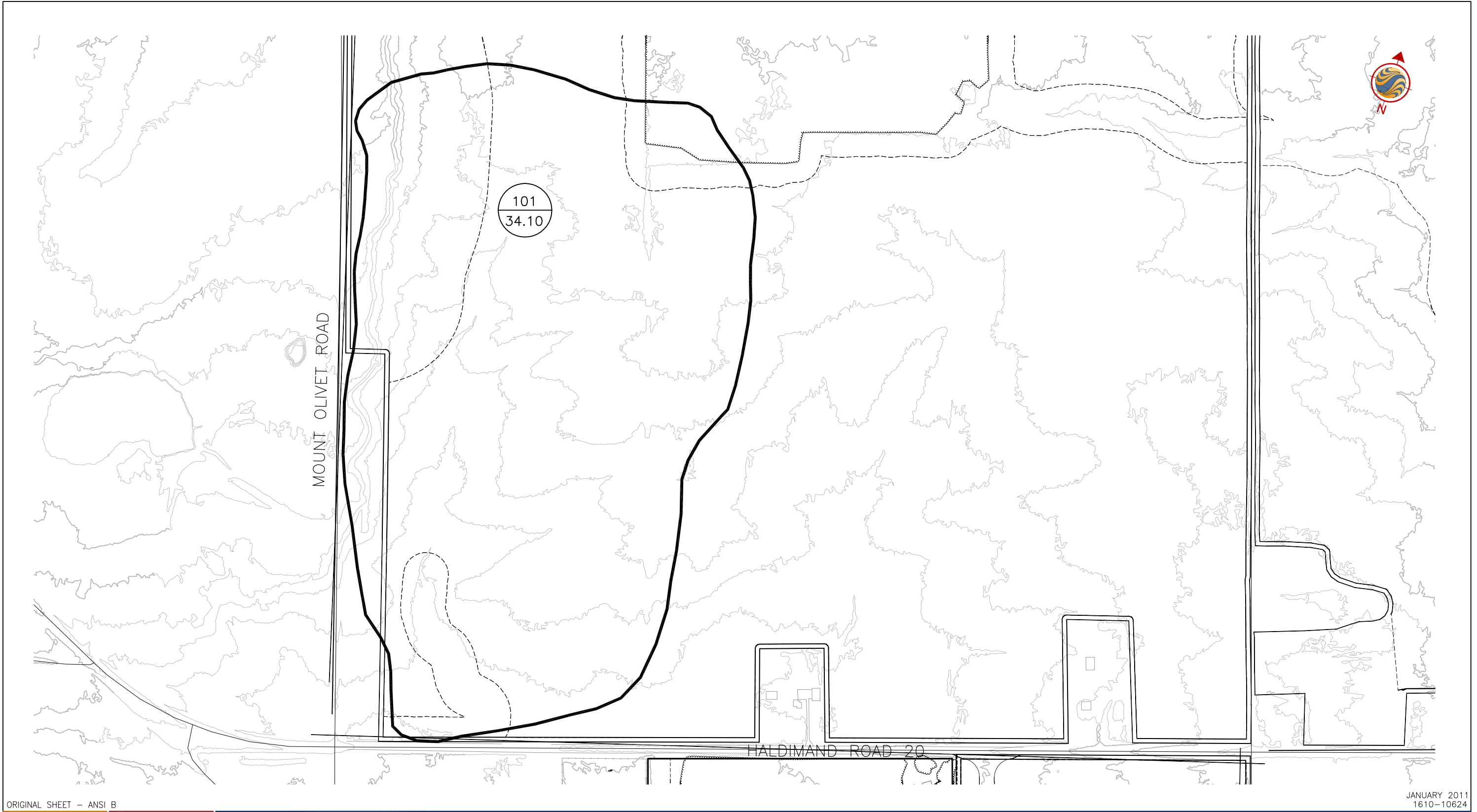
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LOCATION MAP



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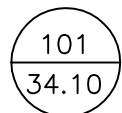
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1610-10624



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Legend



101
34.10
DRAINAGE CATCHMENT NUMBER
CONTRIBUTING AREA (ha)



DRAINAGE CATCHMENT BOUNDARY

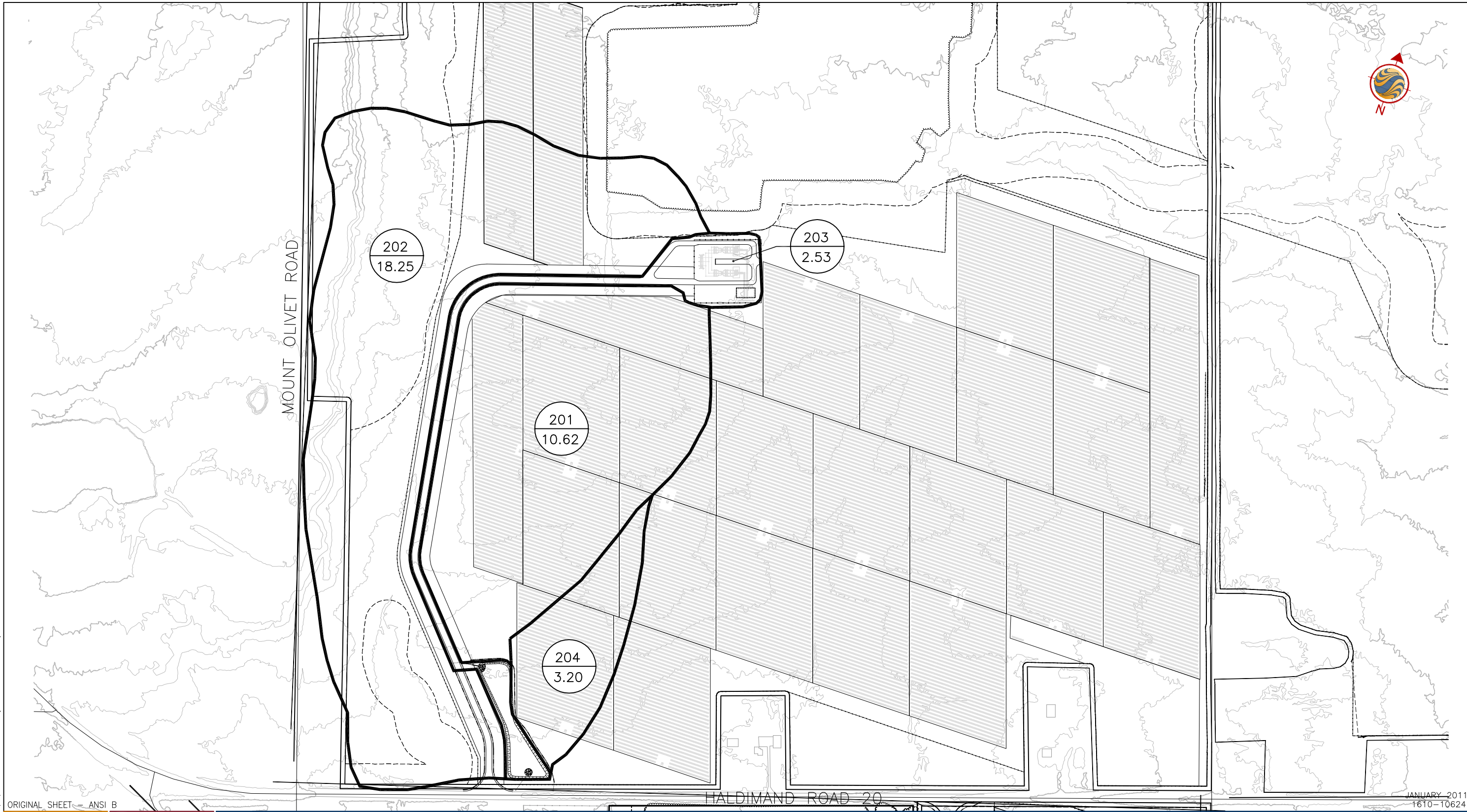


Client/Project
 SAMSUNG RENEWABLE ENERGY INC.
 GRAND RENEWABLE ENERGY PARK
 HALDIMAND, ON

Figure No.
 2.0

Title
**EXISTING CONDITIONS DRAINAGE
 PLAN - SUBSTATION FACILITY**

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-  DRAINAGE CATCHMENT NUMBER
CONTRIBUTING AREA (ha)
-  DRAINAGE CATCHMENT BOUNDARY

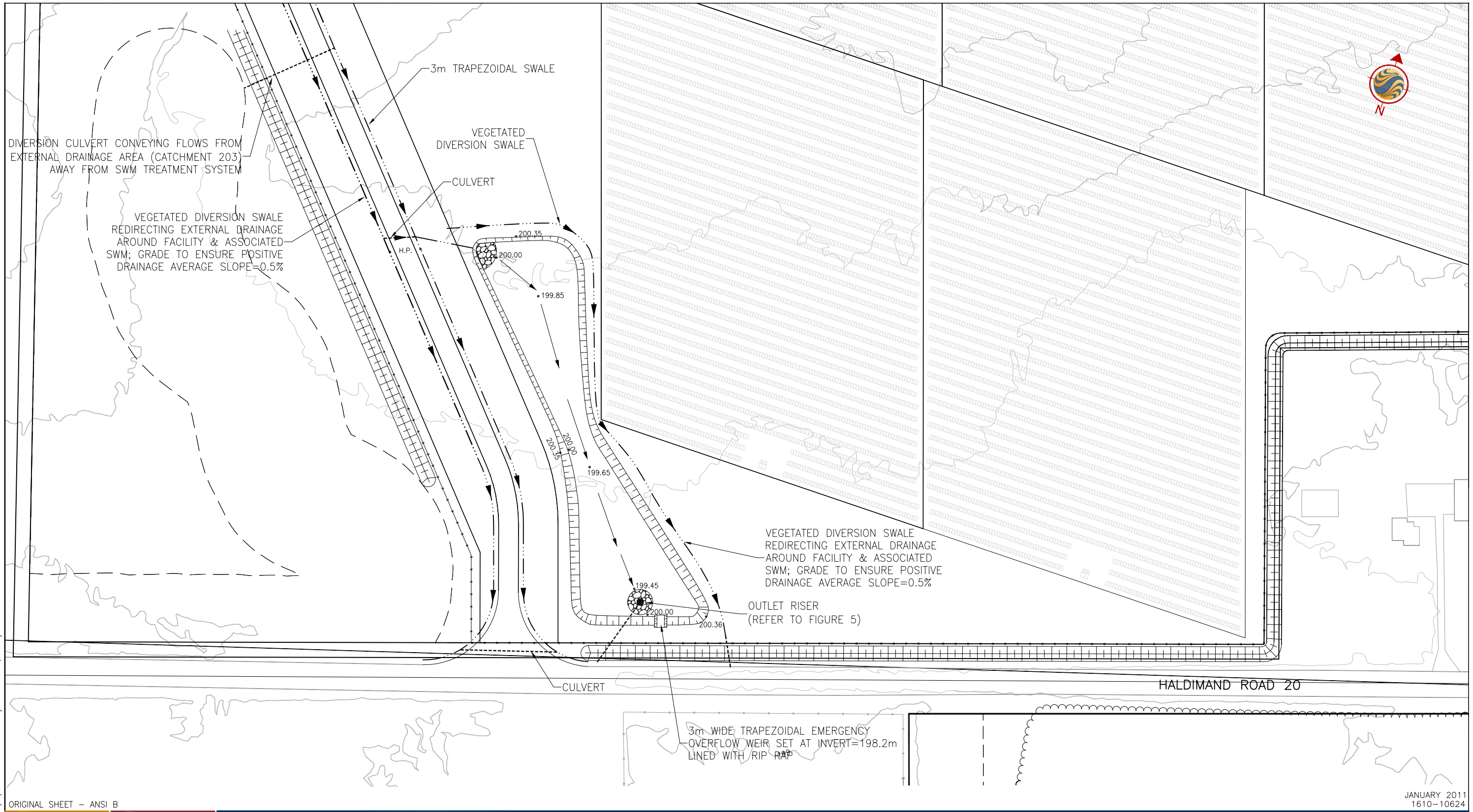


Client/Project
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 GRAND RENEWABLE ENERGY PARK
 HALDIMAND, ON

Figure No.
3.0

Title
**PROPOSED CONDITIONS DRAINAGE
 PLAN - SUBSTATION FACILITY**

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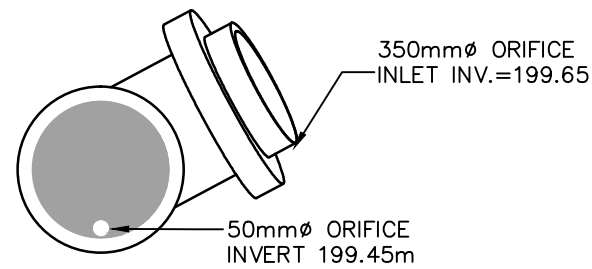
	SLOPE (3:1 UNLESS NOTED OTHERWISE)
	ELEVATION
	CONTOUR
	SWALE



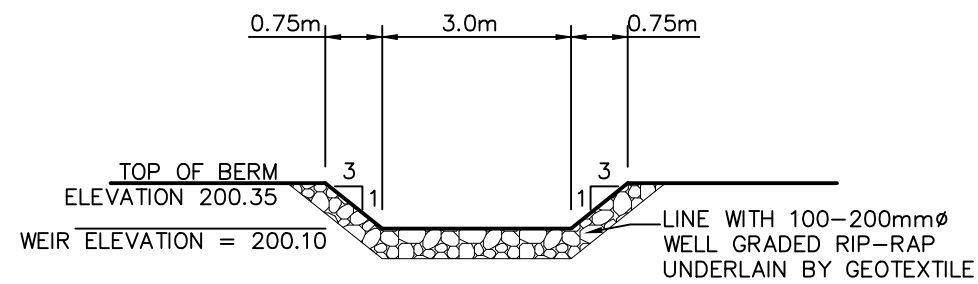
Client/Project
 SAMSUNG RENEWABLE ENERGY INC.
 GRAND RENEWABLE ENERGY PARK
 HALDIMAND, ON

Figure No.
4.0

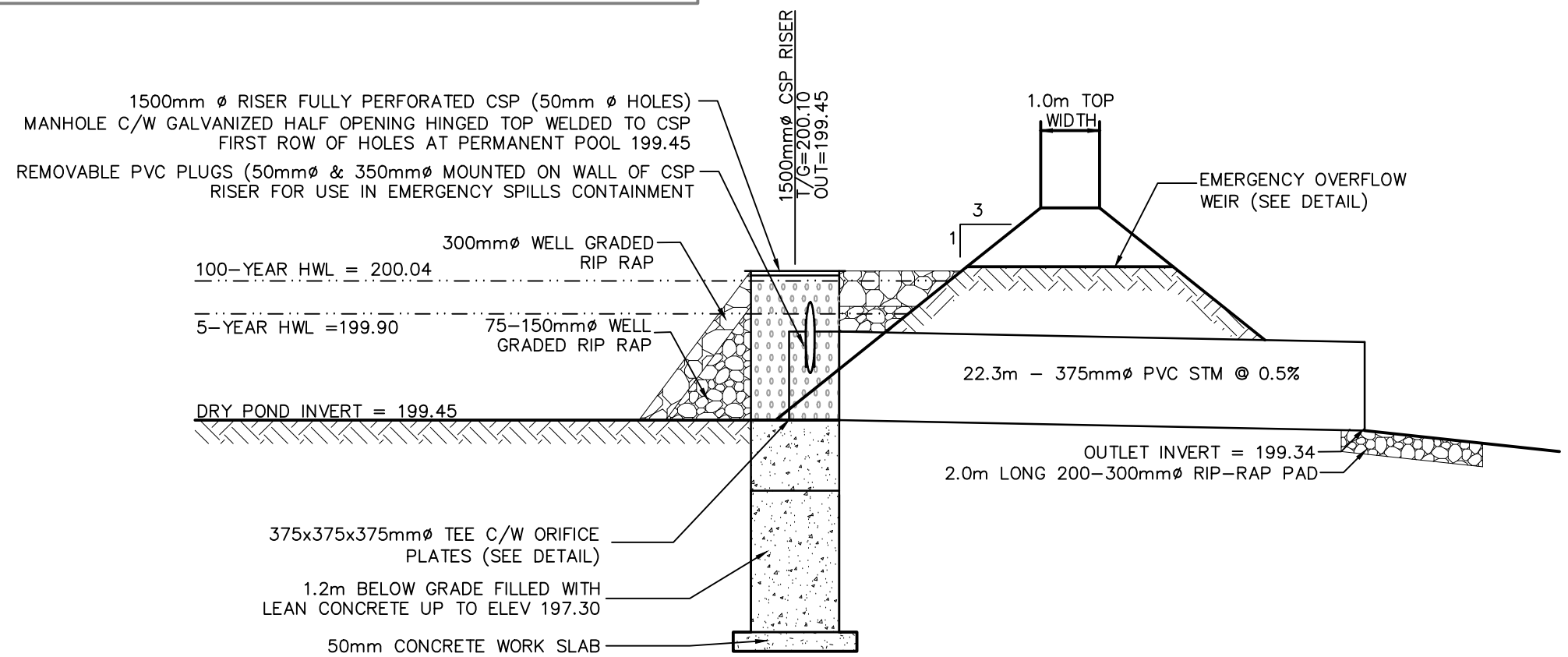
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**SUBSTATION SWM FACILITY -
 PLAN VIEW**



375x375x375 PVC TEE
END VIEW SHOWING ORIENTATION



EMERGENCY WEIR CROSS-SECTION



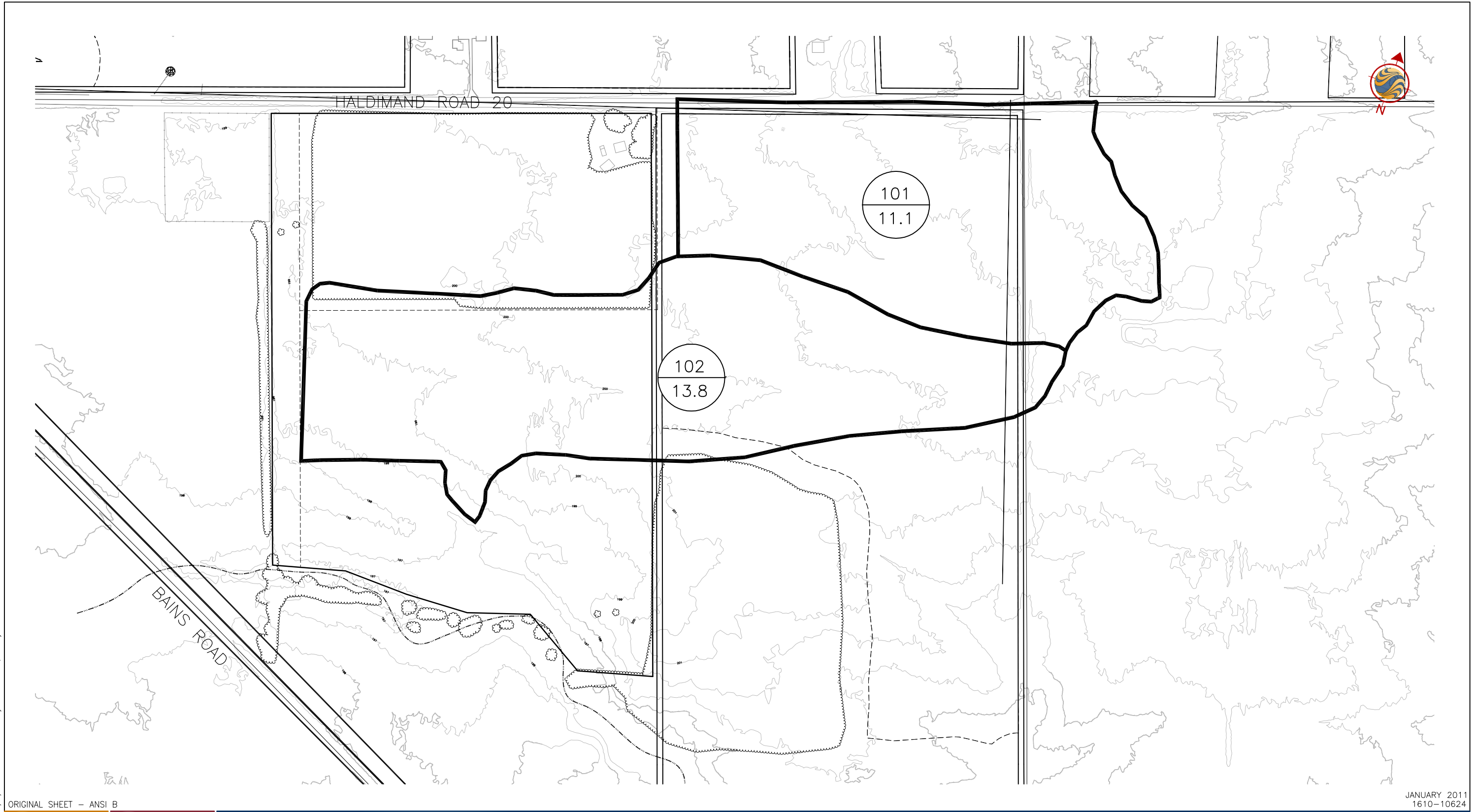
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GRAND RENEWABLE ENERGY PARK
HALDIMAND, ON
Figure No.
5.0
Title
SUBSTATION SWM FACILITY -
OUTLET PROFILE AND DETAIL

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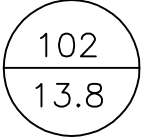

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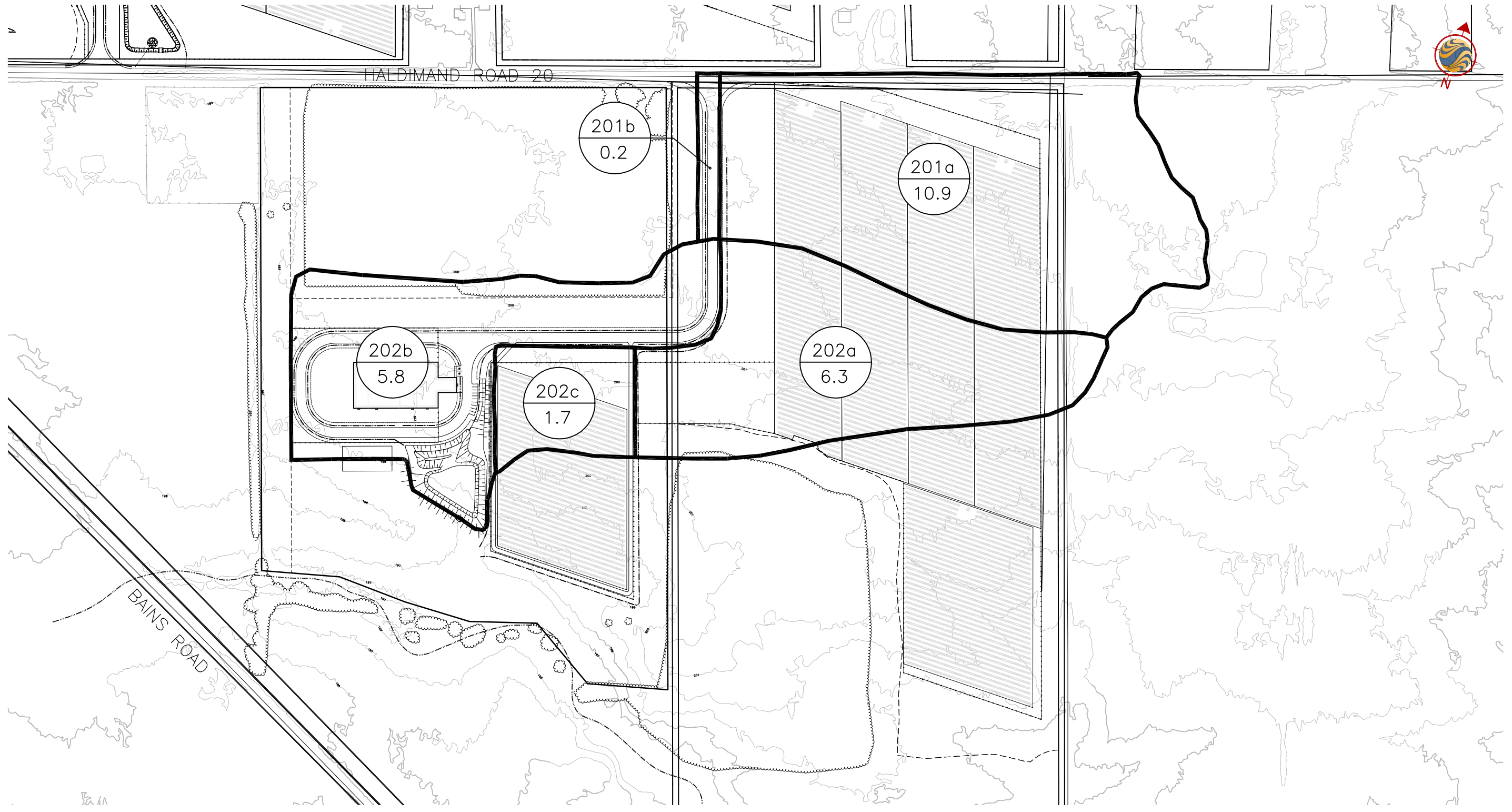
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CONTRIBUTING AREA (ha)
-  DRAINAGE CATCHMENT BOUNDARY



Client/Project
 SAMSUNG RENEWABLE ENERGY INC.
 GRAND RENEWABLE ENERGY PARK
 HALDIMAND, ON

Figure No.
6.0

Title
**EXISTING CONDITIONS DRAINAGE
 PLAN - O & M FACILITY**



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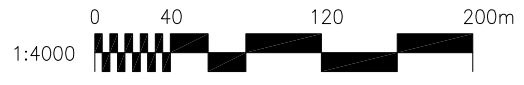
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-  DRAINAGE CATCHMENT NUMBER
CONTRIBUTING AREA (ha)
-  DRAINAGE CATCHMENT BOUNDARY



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SAMSUNG RENEWABLE ENERGY INC.
GRAND RENEWABLE ENERGY PARK
HALDIMAND, ON

Figure No.
7.0

Title
**PROPOSED CONDITIONS DRAINAGE
PLAN - O & M FACILITY**

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**GRAND RENEWABLE ENERGY PARK
STORMWATER MANAGEMENT REPORT**

Appendix B

Substation Analyses

Grand Renewable Energy Park - Substation Facility
Samsung Renewable Energy Inc.
SCS Curve Number Determination
Existing Conditions

Site Soils: (as per Soil Survey Complex, OMAFRA / MNR, 2009)

Soil Type	Hydrologic Soil Group
Hal: Haldimand (clay)	C
Lic: Lincoln (clay)	D

TABLE OF CURVE NUMBERS (CN's)							
Land Use	Hydrologic Soil Type						
	A	AB	B	BC	C	CD	D
Meadow	50	54	58	64.5	71	74.5	78
Woodlot	50	55.3	60.5	67	73.5	76.8	80
Long Grass	55	60	65	72	79	81.5	84
Lawns	60	65.5	71	77	83	86	89
Pasture/Range	58	61.5	65	70.5	76	78.5	81
Crop	66	70	74	78	82	84	86
Fallow (Bare)	77	82	86	89	91	93	94
Wetland	50	50	50	50	50	50	50

HYDROLOGIC SOIL TYPE (%) - Existing Conditions								
Catchment	Hydrologic Soil Type							TOTAL
	A	AB	B	BC	C	CD	D	
101					56		44	100

LAND USE (%) - Existing Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Total
101						100			100

CURVE NUMBER (CN) - Existing Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Weighted CN
101	0	0	0	0	0	84	0	0	83

** post development catchments concerned with pervious CN values only
 ** AMC II assumed
 ** Hydrological Soil Group taken from MTO Drainage Manual for each soil type

Grand Renewable Energy Park - Substation Facility

Samsung Renewable Energy Inc.

SCS Curve Number Determination (Applies to Pervious Component of Developed Catchments Only)

Proposed Conditions

Site Soils: (as per Soil Survey Complex, OMAFRA / MNR, 2009)

Soil Type Hydrologic Soil Group

Hal: Haldimand (clay) C

Lic: Lincoln (clay) D

TABLE OF CURVE NUMBERS (CN's)							
Land Use	Hydrologic Soil Type						
	A	AB	B	BC	C	CD	D
Meadow	50	54	58	65	71	75	78
Woodlot	50	55	61	67	74	77	80
Long Grass	55	60	65	72	79	82	84
Lawns	60	66	71	77	83	86	89
Pasture/Range	58	62	65	71	76	79	81
Crop	66	70	74	78	82	84	86
Fallow (Bare)	77	82	86	89	91	93	94
Wetland	50	50	50	50	50	50	50

HYDROLOGIC SOIL TYPE (%) - Proposed Conditions								
Catchment	Hydrologic Soil Type							TOTAL
	A	AB	B	BC	C	CD	D	
201					68		32	100
202					57		43	100
203					20		80	100
204					85		15	100

LAND USE (%) - Proposed Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Total
201			100						100
202			11			89			100
203			100						100
204			100						100

CURVE NUMBER (CN) - Proposed Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Weighted CN
201	0	0	81	0	0	0	0	0	81
202	0	0	9	0	0	75	0	0	83
203	0	0	83	0	0	0	0	0	83
204	0	0	80	0	0	0	0	0	80

** post development catchments concerned with pervious CN values only

** AMC II assumed

** Hydrological Soil Group taken from MTO Drainage Manual for each soil type

Grand Renewable Energy Park - Substation Facility
Samsung Renewable Energy Inc.
SWMHYMO Parameters

Existing Conditions

Catchment Number	Area Description	SWMHYMO Command	Area (ha)	CN	TIMP	XIMP	Slope (%)	Length (m)	Tc (hrs)	Tp (hrs)
101	Agricultural area just east of Mount Olivet Road containing area of proposed access road	DESIGN NASHYD	34.10	83	5.00	5.00	0.63	1000	1.80	1.08

Proposed Conditions

Catchment Number	Area Description	SWMHYMO Command	Area (ha)	CN	TIMP	XIMP	Slope (%)	Length (m)	Tc (hrs)	Tp (hrs)
201	Solar module field draining to diversion swale on east side of access road	DESIGN NASHYD	10.62	81	0.05	0.05	0.58	950	1.81	1.08
202	Agricultural area + 1 solar module draining to diversion swale on west side of access road	DESIGN NASHYD	18.25	83	0.05	0.05	0.44	1020	2.05	1.23
203	Substation, access road, and grassed swale drainage ditches, and SWM facility	DESIGN STANDHYD	2.53	83	0.60	0.60	0.43	1050	2.10	1.26
204	Solar module area draining to diversion swale on east side of SWM facility	DESIGN NASHYD	3.20	80	0.00	0.00	0.46	650	1.61	0.97

Total area draining to SWMF including SWM Block = 2.53 ha
 Total area draining to SWMF requiring quality control = 2.03 ha 75%

Notes:

CN calculated for pervious areas only for DESIGN STANDHYD. CN is a weighed average for DESIGN NASHYD

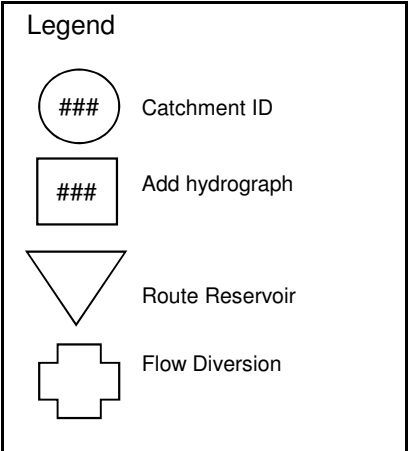
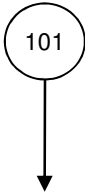
TIMP Total percent impervious

XIMP Percent impervious directly connected

Time of Concentration calculated using the Airport Method $T_c = [3.26 (1.1-C) L^{0.5}] / S^{0.33}$
 Where: C = Runoff Coefficient = 0.2 for undeveloped areas
 L = Length of Overland Flow (m)
 = (Area/1.5)^{0.5}
 S = Slope (%)

Time to Peak $T_p = 0.6T_c$

Grand Renewable Energy Park - Substation Facility
Samsung Renewable Energy Inc.
Existing Conditions SYMHYMO Schematic



```

00001> 2      Metric units
00002> #*****
00003> # Project Name : Grand Renewable Energy Park - Substation Facility
00004> # Project Number: 1610-10624
00005> # Date       : 1-24-2011
00006> # Company    : Stantec Consulting Ltd. (Kitchener)
00007> # Modeller   : George Golding, EIT
00008> # Reviewed / Revised : SRobertson (Jan 31, 2011)
00009> # License #  : 4730904
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00011> # EXISTING CONDITIONS
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00015> *          |-----|
00016> READ STORM STORM_FILENAME=["STORM.001"]
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00018> # Catchment North of Haldimand Road
00019> #-----|
00020> DESIGN NASHYD ID=[1], NHYD=["101"], DT=[5]min, AREA=[34.10](ha),
00021> DWF=[0](cms), CN/C=[83], TP=[1.08]hrs,
00022> RAINFALL=[ , , , ](mm/hr), END=-1
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00003> SSSSS W W M M H H Y Y M M O O 999 999 -----
00004> S W W M M M H H Y Y M M O O 9 9 9 9
00005> SSSSS W W M M M H H H Y Y M M O O ## 9 9 9 9 Ver. 4.02
00006> S W W M M H H Y Y M M O O 9999 9999 July 1999
00007> SSSSS W W M M H H Y Y M M O O 9 9 9
00008> ***** Ottawa, Ontario: (613) 727-5199 *****
00009> StormWater Management Hydrologic Model 999 999 -----
00010>
00011> *****
00012> ***** SWMHYMO-99 Ver/4.02 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHYMO-83 and OTTHYMO-89. *****
00016> *****
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Gatineau, Quebec: (819) 243-6858 *****
00019> ***** E-Mail: swmhyo@jfsa.com *****
00020> *****
00021> *****
00022>
00023> *****
00024> ***** Licensed user: Stantec Consulting Ltd. (Kitchener) *****
00025> ***** Kitchener SERIAL#:4730904 *****
00026> *****
00027> *****
00028> *****
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 15000 *****
00032> ***** Max. number of flow points : 15000 *****
00033> *****
00034> *****
00035> ***** DETAILED OUTPUT *****
00036> *****
00037> *****
00038> ***** DATE: 2011-01-31 TIME: 21:34:35 RUN COUNTER: 001066 *****
00039> *****
00040> * Input filename: C:\PROGRA-1\SWMHYMO\Sub_Ex.dat *
00041> * Output filename: C:\PROGRA-1\SWMHYMO\Sub_Ex.out *
00042> * Summary filename: C:\PROGRA-1\SWMHYMO\Sub_Ex.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047> *****
00048> *****
00049> *****
00050> 001:0001-----
00051> # *****
00052> # Project Name : Grand Renewable Energy Park - Substation Facility
00053> # Project Number: 1610-10624
00054> # Date : 1-24-2011
00055> # Company : Stantec Consulting Ltd. (Kitchener)
00056> # Modeller : George Golding, EIT
00057> # Reviewed / Revised : SRobertson (Jan 31, 2011)
00058> # License # : 4730904
00059> *****
00060> # EXISTING CONDITIONS
00061> # *****
00062> # *****
00063> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00064> | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00065> | TZERO = .00 hrs on 0
00066> | METOUT= 2 (output = METRIC)
00067> | NRUN = 001
00068> | NSTORM= 1
00069> | # 1=si_2.STM
00070> |
00071> 001:0002-----
00072> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_2.STM
00073> | Ptotal= 50.70 mm | Comments: SCS-II 24H 2-YEAR SIMCOE
00074> |
00075> |
00076> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00077> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00078> | .25 .558 | 6.25 1.014 | 12.25 7.301 | 18.25 .913
00079> | .50 .558 | 6.50 1.014 | 12.50 7.301 | 18.50 .913
00080> | .75 .558 | 6.75 1.014 | 12.75 3.752 | 18.75 .913
00081> | 1.00 .558 | 7.00 1.014 | 13.00 3.752 | 19.00 .913
00082> | 1.25 .558 | 7.25 1.014 | 13.25 2.636 | 19.25 .913
00083> | 1.50 .558 | 7.50 1.014 | 13.50 2.839 | 19.50 .913
00084> | 1.75 .558 | 7.75 1.014 | 13.75 2.129 | 19.75 .913
00085> | 2.00 .558 | 8.00 1.014 | 14.00 2.129 | 20.00 .913
00086> | 2.25 .659 | 8.25 1.369 | 14.25 1.521 | 20.25 .608
00087> | 2.50 .659 | 8.50 1.369 | 14.50 1.521 | 20.50 .608
00088> | 2.75 .659 | 8.75 1.369 | 14.75 1.521 | 20.75 .608
00089> | 3.00 .659 | 9.00 1.369 | 15.00 1.521 | 21.00 .608
00090> | 3.25 .659 | 9.25 1.622 | 15.25 1.521 | 21.25 .608
00091> | 3.50 .659 | 9.50 1.622 | 15.50 1.521 | 21.50 .608
00092> | 3.75 .659 | 9.75 1.825 | 15.75 1.521 | 21.75 .608
00093> | 4.00 .659 | 10.00 1.825 | 16.00 1.521 | 22.00 .608
00094> | 4.25 .811 | 10.25 2.332 | 16.25 .913 | 22.25 .608
00095> | 4.50 .811 | 10.50 2.332 | 16.50 .913 | 22.50 .608
00096> | 4.75 .811 | 10.75 3.143 | 16.75 .913 | 22.75 .608
00097> | 5.00 .811 | 11.00 3.143 | 17.00 .913 | 23.00 .608
00098> | 5.25 .811 | 11.25 4.867 | 17.25 .913 | 23.25 .608
00099> | 5.50 .811 | 11.50 4.867 | 17.50 .913 | 23.50 .608
00100> | 5.75 .811 | 11.75 21.091 | 17.75 .913 | 23.75 .608
00101> | 6.00 .811 | 12.00 55.973 | 18.00 .913 | 24.00 .608
00102>
00103>
00104> 001:0003-----
00105> # *****
00106> # Catchment North of Haldimand Road
00107> # *****
00108> # *****
00109> | DESIGN NASHYD | Area (ha)= 34.10 Curve Number (CN)=83.00
00110> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00111> | U.H. Tp(hrs)= 1.080
00112> |
00113> | Unit Hyd Qpeak (cms)= 1.206
00114> |
00115> | PEAK FLOW (cms)= .613 (i)
00116> | TIME TO PEAK (hrs)= 13.000
00117> | RUNOFF VOLUME (mm)= 23.913
00118> | TOTAL RAINFALL (mm)= 50.699
00119> | RUNOFF COEFFICIENT = .472
00120> |
00121> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00122> |
00123> |
00124> 001:0004-----
00125> # *****
00126> ** END OF RUN : 1
00127> *****
00128> *****
00129> *****
00130> *****
00131> *****
00132> *****
00133> *****
00134> *****
00135> | START | Project dir.: C:\PROGRA-1\SWMHYMO\

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00136>----- Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00137> TZERO = .00 hrs on 0
00138> METOUT= 2 (output = METRIC)
00139> NRUN = 002
00140> NSTORM= 1
00141> # 1=si_5.STM
00142>-----
00143>-----
00144> *****
00145> # Project Name : Grand Renewable Energy Park - Substation Facility
00146> # Project Number: 1610-10624
00147> # Date : 1-24-2011
00148> # Company : Stantec Consulting Ltd. (Kitchener)
00149> # Modeller : George Golding, EIT
00150> # Reviewed / Revised : SRobertson (Jan 31, 2011)
00151> # License # : 4730904
00152> *****
00153> # EXISTING CONDITIONS
00154> # *****
00155> # *****
00156> 002:0002-----
00157> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_5.STM
00158> | Ptotal= 66.80 mm | Comments: SCS-II 24H 5-YEAR SIMCOE
00159> |
00160> |
00161> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00162> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00163> | .25 .735 | 6.25 1.336 | 12.25 9.619 | 18.25 1.202
00164> | .50 .735 | 6.50 1.336 | 12.50 9.619 | 18.50 1.202
00165> | .75 .735 | 6.75 1.336 | 12.75 4.943 | 18.75 1.202
00166> | 1.00 .735 | 7.00 1.336 | 13.00 4.943 | 19.00 1.202
00167> | 1.25 .735 | 7.25 1.336 | 13.25 3.474 | 19.25 1.202
00168> | 1.50 .735 | 7.50 1.336 | 13.50 3.741 | 19.50 1.202
00169> | 1.75 .735 | 7.75 1.336 | 13.75 2.806 | 19.75 1.202
00170> | 2.00 .735 | 8.00 1.336 | 14.00 2.806 | 20.00 1.202
00171> | 2.25 .868 | 8.25 1.804 | 14.25 2.004 | 20.25 .802
00172> | 2.50 .868 | 8.50 1.804 | 14.50 2.004 | 20.50 .802
00173> | 2.75 .868 | 8.75 1.804 | 14.75 2.004 | 20.75 .802
00174> | 3.00 .868 | 9.00 1.804 | 15.00 2.004 | 21.00 .802
00175> | 3.25 .868 | 9.25 2.138 | 15.25 2.004 | 21.25 .802
00176> | 3.50 .868 | 9.50 2.138 | 15.50 2.004 | 21.50 .802
00177> | 3.75 .868 | 9.75 2.405 | 15.75 2.004 | 21.75 .802
00178> | 4.00 .868 | 10.00 2.405 | 16.00 2.004 | 22.00 .802
00179> | 4.25 1.069 | 10.25 3.073 | 16.25 1.202 | 22.25 .802
00180> | 4.50 1.069 | 10.50 3.073 | 16.50 1.202 | 22.50 .802
00181> | 4.75 1.069 | 10.75 4.142 | 16.75 1.202 | 22.75 .802
00182> | 5.00 1.069 | 11.00 4.142 | 17.00 1.202 | 23.00 .802
00183> | 5.25 1.069 | 11.25 6.413 | 17.25 1.202 | 23.25 .802
00184> | 5.50 1.069 | 11.50 6.413 | 17.50 1.202 | 23.50 .802
00185> | 5.75 1.069 | 11.75 27.789 | 17.75 1.202 | 23.75 .802
00186> | 6.00 1.069 | 12.00 73.747 | 18.00 1.202 | 24.00 .802
00187>
00188>
00189> 002:0003-----
00190> # *****
00191> # Catchment North of Haldimand Road
00192> # *****
00193> # *****
00194> | DESIGN NASHYD | Area (ha)= 34.10 Curve Number (CN)=83.00
00195> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00196> | U.H. Tp(hrs)= 1.080
00197> |
00198> | Unit Hyd Qpeak (cms)= 1.206
00199> |
00200> | PEAK FLOW (cms)= .941 (i)
00201> | TIME TO PEAK (hrs)= 13.000
00202> | RUNOFF VOLUME (mm)= 36.346
00203> | TOTAL RAINFALL (mm)= 66.801
00204> | RUNOFF COEFFICIENT = .544
00205> |
00206> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00207> |
00208> |
00209> 002:0004-----
00210> # *****
00211> # *****
00212> 002:0002-----
00213> ** END OF RUN : 2
00214> *****
00215> *****
00216> *****
00217> *****
00218> *****
00219> *****
00220> *****
00221> *****
00222> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00223> | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00224> | TZERO = .00 hrs on 0
00225> | METOUT= 2 (output = METRIC)
00226> | NRUN = 003
00227> | NSTORM= 1
00228> | # 1=si_10.STM
00229> |
00230> 003:0002-----
00231> # *****
00232> # Project Name : Grand Renewable Energy Park - Substation Facility
00233> # Project Number: 1610-10624
00234> # Date : 1-24-2011
00235> # Company : Stantec Consulting Ltd. (Kitchener)
00236> # Modeller : George Golding, EIT
00237> # Reviewed / Revised : SRobertson (Jan 31, 2011)
00238> # License # : 4730904
00239> *****
00240> # EXISTING CONDITIONS
00241> # *****
00242> # *****
00243> 003:0002-----
00244> # *****
00245> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_10.STM
00246> | Ptotal= 77.40 mm | Comments: SCS-II 24H 10-YEAR SIMCOE
00247> |
00248> |
00249> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00250> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00251> | .25 .851 | 6.25 1.548 | 12.25 11.146 | 18.25 1.393
00252> | .50 .851 | 6.50 1.548 | 12.50 11.146 | 18.50 1.393
00253> | .75 .851 | 6.75 1.548 | 12.75 5.728 | 18.75 1.393
00254> | 1.00 .851 | 7.00 1.548 | 13.00 5.728 | 19.00 1.393
00255> | 1.25 .851 | 7.25 1.548 | 13.25 4.025 | 19.25 1.393
00256> | 1.50 .851 | 7.50 1.548 | 13.50 4.334 | 19.50 1.393
00257> | 1.75 .851 | 7.75 1.548 | 13.75 3.251 | 19.75 1.393
00258> | 2.00 .851 | 8.00 1.548 | 14.00 3.251 | 20.00 1.393
00259> | 2.25 1.006 | 8.25 2.090 | 14.25 2.322 | 20.25 .929
00260> | 2.50 1.006 | 8.50 2.090 | 14.50 2.322 | 20.50 .929
00261> | 2.75 1.006 | 8.75 2.090 | 14.75 2.322 | 20.75 .929
00262> | 3.00 1.006 | 9.00 2.090 | 15.00 2.322 | 21.00 .929
00263> | 3.25 1.006 | 9.25 2.477 | 15.25 2.322 | 21.25 .929
00264> | 3.50 1.006 | 9.50 2.477 | 15.50 2.322 | 21.50 .929
00265> | 3.75 1.006 | 9.75 2.786 | 15.75 2.322 | 21.75 .929
00266> | 4.00 1.006 | 10.00 2.786 | 16.00 2.322 | 22.00 .929
00267> | 4.25 1.238 | 10.25 3.560 | 16.25 1.393 | 22.25 .929
00268> | 4.50 1.238 | 10.50 4.799 | 16.75 1.393 | 22.75 .929
00269> | 5.00 1.238 | 11.00 4.799 | 17.00 1.393 | 23.00 .929
00270> | 5.25 1.238 | 11.25 7.430 | 17.25 1.393 | 23.25 .929

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00271> 5.50 1.238 | 11.50 7.430 | 17.50 1.393 | 23.50 .929
00272> 5.75 1.238 | 11.75 32.198 | 17.75 1.393 | 23.75 .929
00273> 6.00 1.238 | 12.00 85.450 | 18.00 1.393 | 24.00 .929
00274>
00275>
00276> 003:0003-----
00277> *#-----
00278> *# Catchment North of Haldimand Road
00279> *#-----
00280>
00281> | DESIGN NASHYD | Area (ha)= 34.10 Curve Number (CN)=83.00
00282> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00283> | U.H. Tp(hrs)= 1.080
00284>
00285> Unit Hyd Qpeak (cms)= 1.206
00286>
00287> PEAK FLOW (cms)= 1.171 (i)
00288> TIME TO PEAK (hrs)= 13.000
00289> RUNOFF VOLUME (mm)= 45.032
00290> TOTAL RAINFALL (mm)= 77.398
00291> RUNOFF COEFFICIENT = .582
00292>
00293> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00294>
00295>
00296> 003:0004-----
00297> *#-----
00298>
00299> 003:0002-----
00300>
00301> 003:0002-----
00302> ** END OF RUN : 3
00303>
00304> *****
00305>
00306>
00307>
00308>
00309>
00310>
00311> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00312> | TZERO = .00 hrs on | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00313> | METOUT= 2 (output = METRIC)
00314> | NRUN = 004
00315> | NSTORM= 1
00316> | # 1=si_25.STM
00317>
00318>
00319> 004:0002-----
00320> *#-----
00321> *# Project Name : Grand Renewable Energy Park - Substation Facility
00322> *# Project Number: 1610-10624
00323> *# Date : 1-24-2011
00324> *# Company : Stantec Consulting Ltd. (Kitchener)
00325> *# Modeller : George Golding, EIT
00326> *# Reviewed / Revised : SRobertson (Jan 31, 2011)
00327> *# License # : 4730904
00328> *#-----
00329> *# EXISTING CONDITIONS
00330> *#-----
00331>
00332> 004:0002-----
00333>
00334> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_25.STM
00335> | Ptotal= 90.80 mm | Comments: SCS-II 24H 25-YEAR SIMCOE
00336>
00337> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00338> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00339> .25 .999 | 6.25 1.816 | 12.25 13.075 | 18.25 1.634
00340> .50 .999 | 6.50 1.816 | 12.50 13.075 | 18.50 1.634
00341> .75 .999 | 6.75 1.816 | 12.75 6.719 | 18.75 1.634
00342> 1.00 .999 | 7.00 1.816 | 13.00 6.719 | 19.00 1.634
00343> 1.25 .999 | 7.25 1.816 | 13.25 4.722 | 19.25 1.634
00344> 1.50 .999 | 7.50 1.816 | 13.50 5.085 | 19.50 1.634
00345> 1.75 .999 | 7.75 1.816 | 13.75 3.814 | 19.75 1.634
00346> 2.00 .999 | 8.00 1.816 | 14.00 3.814 | 20.00 1.634
00347> 2.25 1.180 | 8.25 2.452 | 14.25 2.724 | 20.25 1.090
00348> 2.50 1.180 | 8.50 2.452 | 14.50 2.724 | 20.50 1.090
00349> 2.75 1.180 | 8.75 2.452 | 14.75 2.724 | 20.75 1.090
00350> 3.00 1.180 | 9.00 2.452 | 15.00 2.724 | 21.00 1.090
00351> 3.25 1.180 | 9.25 2.906 | 15.25 2.724 | 21.25 1.090
00352> 3.50 1.180 | 9.50 2.906 | 15.50 2.724 | 21.50 1.090
00353> 3.75 1.180 | 9.75 3.269 | 15.75 2.724 | 21.75 1.090
00354> 4.00 1.180 | 10.00 3.269 | 16.00 2.724 | 22.00 1.090
00355> 4.25 1.453 | 10.25 4.177 | 16.25 1.634 | 22.25 1.090
00356> 4.50 1.453 | 10.50 4.177 | 16.50 1.634 | 22.50 1.090
00357> 4.75 1.453 | 10.75 5.630 | 16.75 1.634 | 22.75 1.090
00358> 5.00 1.453 | 11.00 5.630 | 17.00 1.634 | 23.00 1.090
00359> 5.25 1.453 | 11.25 8.717 | 17.25 1.634 | 23.25 1.090
00360> 5.50 1.453 | 11.50 8.717 | 17.50 1.634 | 23.50 1.090
00361> 5.75 1.453 | 11.75 37.773 | 17.75 1.634 | 23.75 1.090
00362> 6.00 1.453 | 12.00 100.243 | 18.00 1.634 | 24.00 1.090
00363>
00364>
00365> 004:0003-----
00366> *#-----
00367> *# Catchment North of Haldimand Road
00368> *#-----
00369>
00370> | DESIGN NASHYD | Area (ha)= 34.10 Curve Number (CN)=83.00
00371> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00372> | U.H. Tp(hrs)= 1.080
00373>
00374> Unit Hyd Qpeak (cms)= 1.206
00375>
00376> PEAK FLOW (cms)= 1.473 (i)
00377> TIME TO PEAK (hrs)= 13.000
00378> RUNOFF VOLUME (mm)= 56.428
00379> TOTAL RAINFALL (mm)= 90.801
00380> RUNOFF COEFFICIENT = .621
00381>
00382> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00383>
00384>
00385> 004:0004-----
00386> *#-----
00387>
00388> 004:0002-----
00389>
00390> 004:0002-----
00391>
00392> 004:0002-----
00393> ** END OF RUN : 4
00394>
00395> *****
00396>
00397>
00398>
00399>
00400>
00401>
00402> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00403> | TZERO = .00 hrs on | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00404> | METOUT= 2 (output = METRIC)
00405>

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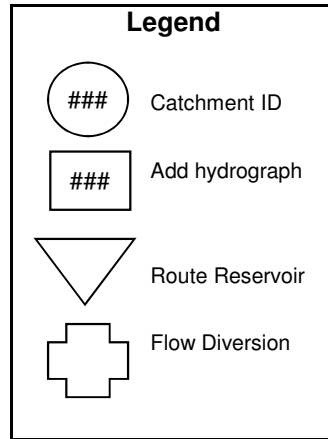
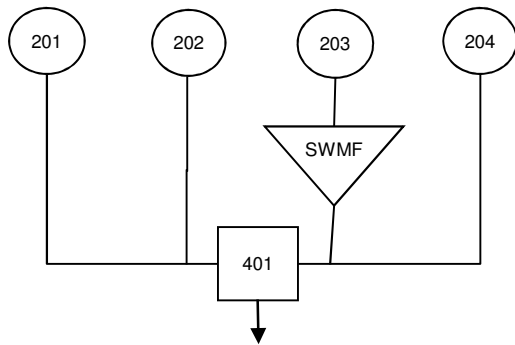
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00406> NRUN = 005
00407> NSTORM= 1
00408> # 1=si_50.STM
00409>
00410> 005:0002-----
00411> *#-----
00412> *# Project Name : Grand Renewable Energy Park - Substation Facility
00413> *# Project Number: 1610-10624
00414> *# Date : 1-24-2011
00415> *# Company : Stantec Consulting Ltd. (Kitchener)
00416> *# Modeller : George Golding, EIT
00417> *# Reviewed / Revised : SRobertson (Jan 31, 2011)
00418> *# License # : 4730904
00419> *#-----
00420> *# EXISTING CONDITIONS
00421> *#-----
00422>
00423> 005:0002-----
00424>
00425> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_50.STM
00426> | Ptotal= 100.70 mm | Comments: SCS-II 24H 50-YEAR SIMCOE
00427>
00428> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00429> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00430> .25 1.108 | 6.25 2.014 | 12.25 14.501 | 18.25 1.813
00431> .50 1.108 | 6.50 2.014 | 12.50 14.501 | 18.50 1.813
00432> .75 1.108 | 6.75 2.014 | 12.75 7.452 | 18.75 1.813
00433> 1.00 1.108 | 7.00 2.014 | 13.00 7.452 | 19.00 1.813
00434> 1.25 1.108 | 7.25 2.014 | 13.25 5.236 | 19.25 1.813
00435> 1.50 1.108 | 7.50 2.014 | 13.50 5.639 | 19.50 1.813
00436> 1.75 1.108 | 7.75 2.014 | 13.75 4.229 | 19.75 1.813
00437> 2.00 1.108 | 8.00 2.014 | 14.00 4.229 | 20.00 1.813
00438> 2.25 1.309 | 8.25 2.719 | 14.25 3.021 | 20.25 1.208
00439> 2.50 1.309 | 8.50 2.719 | 14.50 3.021 | 20.50 1.208
00440> 2.75 1.309 | 8.75 2.719 | 14.75 3.021 | 20.75 1.208
00441> 3.00 1.309 | 9.00 2.719 | 15.00 3.021 | 21.00 1.208
00442> 3.25 1.309 | 9.25 3.222 | 15.25 3.021 | 21.25 1.208
00443> 3.50 1.309 | 9.50 3.222 | 15.50 3.021 | 21.50 1.208
00444> 3.75 1.309 | 9.75 3.625 | 15.75 3.021 | 21.75 1.208
00445> 4.00 1.309 | 10.00 3.625 | 16.00 3.021 | 22.00 1.208
00446> 4.25 1.611 | 10.25 4.632 | 16.25 1.813 | 22.25 1.208
00447> 4.50 1.611 | 10.50 4.632 | 16.50 1.813 | 22.50 1.208
00448> 4.75 1.611 | 10.75 6.243 | 16.75 1.813 | 22.75 1.208
00449> 5.00 1.611 | 11.00 6.243 | 17.00 1.813 | 23.00 1.208
00450> 5.25 1.611 | 11.25 9.667 | 17.25 1.813 | 23.25 1.208
00451> 5.50 1.611 | 11.50 9.667 | 17.50 1.813 | 23.50 1.208
00452> 5.75 1.611 | 11.75 41.891 | 17.75 1.813 | 23.75 1.208
00453> 6.00 1.611 | 12.00 111.173 | 18.00 1.813 | 24.00 1.208
00454>
00455>
00456> 005:0003-----
00457> *#-----
00458> *# Catchment North of Haldimand Road
00459> *#-----
00460>
00461> | DESIGN NASHYD | Area (ha)= 34.10 Curve Number (CN)=83.00
00462> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00463> | U.H. Tp(hrs)= 1.080
00464>
00465> Unit Hyd Qpeak (cms)= 1.206
00466>
00467> PEAK FLOW (cms)= 1.702 (i)
00468> TIME TO PEAK (hrs)= 13.000
00469> RUNOFF VOLUME (mm)= 65.073
00470> TOTAL RAINFALL (mm)= 100.699
00471> RUNOFF COEFFICIENT = .646
00472>
00473> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00474>
00475>
00476> 005:0004-----
00477> *#-----
00478>
00479> 005:0002-----
00480>
00481> 005:0002-----
00482>
00483>
00484>
00485> 005:0002-----
00486> ** END OF RUN : 5
00487>
00488> *****
00489>
00490>
00491>
00492>
00493>
00494>
00495> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00496> | TZERO = .00 hrs on | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00497> | METOUT= 2 (output = METRIC)
00498> | NRUN = 006
00499> | NSTORM= 1
00500> | # 1=si_100.STM
00501>
00502>
00503> 006:0002-----
00504> *#-----
00505> *# Project Name : Grand Renewable Energy Park - Substation Facility
00506> *# Project Number: 1610-10624
00507> *# Date : 1-24-2011
00508> *# Company : Stantec Consulting Ltd. (Kitchener)
00509> *# Modeller : George Golding, EIT
00510> *# Reviewed / Revised : SRobertson (Jan 31, 2011)
00511> *# License # : 4730904
00512> *#-----
00513> *# EXISTING CONDITIONS
00514> *#-----
00515>
00516> 006:0002-----
00517>
00518> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_100.STM
00519> | Ptotal= 110.60 mm | Comments: SCS-II 24H 100-YEAR SIMCOE
00520>
00521> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00522> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00523> .25 1.217 | 6.25 2.212 | 12.25 15.926 | 18.25 1.991
00524> .50 1.217 | 6.50 2.212 | 12.50 15.926 | 18.50 1.991
00525> .75 1.217 | 6.75 2.212 | 12.75 8.184 | 18.75 1.991
00526> 1.00 1.217 | 7.00 2.212 | 13.00 8.184 | 19.00 1.991
00527> 1.25 1.217 | 7.25 2.212 | 13.25 5.751 | 19.25 1.991
00528> 1.50 1.217 | 7.50 2.212 | 13.50 6.194 | 19.50 1.991
00529> 1.75 1.217 | 7.75 2.212 | 13.75 4.645 | 19.75 1.991
00530> 2.00 1.217 | 8.00 2.212 | 14.00 4.645 | 20.00 1.991
00531> 2.25 1.438 | 8.25 2.986 | 14.25 3.318 | 20.25 1.327
00532> 2.50 1.438 | 8.50 2.986 | 14.50 3.318 | 20.50 1.327
00533> 2.75 1.438 | 8.75 2.986 | 14.75 3.318 | 20.75 1.327
00534> 3.00 1.438 | 9.00 2.986 | 15.00 3.318 | 21.00 1.327
00535> 3.25 1.438 | 9.25 3.539 | 15.25 3.318 | 21.25 1.327
00536> 3.50 1.438 | 9.50 3.539 | 15.50 3.318 | 21.50 1.327
00537> 3.75 1.438 | 9.75 3.982 | 15.75 3.318 | 21.75 1.327
00538> 4.00 1.438 | 10.00 3.982 | 16.00 3.318 | 22.00 1.327
00539> 4.25 1.770 | 10.25 5.088 | 16.25 1.991 | 22.25 1.327
00540> 4.50 1.770 | 10.50 5.088 | 16.50 1.991 | 22.50 1.327

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00541>      4.75  1.770 | 10.75  6.857 | 16.75  1.991 | 22.75  1.327
00542>      5.00  1.770 | 11.00  6.857 | 17.00  1.991 | 23.00  1.327
00543>      5.25  1.770 | 11.25  10.618 | 17.25  1.991 | 23.25  1.327
00544>      5.50  1.770 | 11.50  10.618 | 17.50  1.991 | 23.50  1.327
00545>      5.75  1.770 | 11.75  46.010 | 17.75  1.991 | 23.75  1.327
00546>      6.00  1.770 | 12.00 122.102 | 18.00  1.991 | 24.00  1.327
00547>
00548>
00549> 006:0003-----
00550> *#-----|-----|
00551> *# Catchment North of Haldimand Road
00552> *#-----|-----|
00553>-----
00554> | DESIGN NASHYD | Area (ha)= 34.10 Curve Number (CN)=83.00
00555> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00556>-----|-----|
00557> | U.H. Tp(hrs)= 1.080
00558> Unit Hyd Qpeak (cms)= 1.206
00559>
00560> PEAK FLOW (cms)= 1.934 (i)
00561> TIME TO PEAK (hrs)= 13.000
00562> RUNOFF VOLUME (mm)= 73.875
00563> TOTAL RAINFALL (mm)= 110.602
00564> RUNOFF COEFFICIENT = .668
00565>
00566> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00567>
00568>
00569> 006:0004-----
00570> *#-----|-----|
00571>
00572> 006:0002-----
00573>
00574> 006:0002-----
00575>
00576> 006:0002-----
00577>
00578> 006:0002-----
00579>
00580> 006:0002-----
00581> FINISH
00582>-----
00583> *****
00584> WARNINGS / ERRORS / NOTES
00585>-----
00586> Simulation ended on 2011-01-31 at 21:34:36
00587>-----
00588>
```

Grand Renewable Energy Park - Substation Facility
Samsung Renewable Energy Inc.
Proposed Conditions SYMHYMO Schematic



```

00001> 2 Metric units
00002> #*****
00003> # Project Name : Grand Renewable Energy Park-Substation
00004> # Project Number: 1610-10624
00005> # Date : 1-24-2011
00006> # Company : Stantec Consulting Ltd. (Kitchener)
00007> # Modeller : George Golding, EIT
00008> # Reviewed / Revised : S Robertson (Jan 31, 2011)
00009> # License # : 4730904
00010> #*****
00011> # PROPOSED CONDITIONS
00012> #*****
00013> START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[1]
00014> * ["si_2.STM"] <--storm filename, one per line for NSTORM time
00015> *-----|
00016> READ STORM STORM_FILENAME=["STORM.001"]
00017> #-----|
00018> # Catchment North of Haldimand Road
00019> #-----|
00020> DESIGN NASHYD ID=[1], NHYD=["201"], DT=[5]min, AREA=[10.62] (ha),
00021> DWF=[0] (cms), CN/C=[81], TP=[1.08]hrs,
00022> RAINFALL=[ , , , ](mm/hr), END=-1
00023> #-----|
00024> # Catchment North of Haldimand Road
00025> #-----|
00026> DESIGN NASHYD ID=[2], NHYD=["202"], DT=[5]min, AREA=[18.25] (ha),
00027> DWF=[0] (cms), CN/C=[83], TP=[1.23]hrs,
00028> RAINFALL=[ , , , ](mm/hr), END=-1
00029> #-----|
00030> # Catchment North of Haldimand Road
00031> #-----|
00032> DESIGN STANDHYD ID=[3], NHYD=["203"], DT=[1]min, AREA=[2.53] (ha),
00033> XIMP=[0.60], TMP=[0.60], DWF=[0] (cms), LOSS=[2], CN=[83],
00034> SLOPE=[0.43](%), RAINFALL=[ , , , ](mm/hr), END=-1
00035> #-----|
00036> # Catchment North of Haldimand Road
00037> #-----|
00038> DESIGN NASHYD ID=[4], NHYD=["204"], DT=[5]min, AREA=[3.20] (ha),
00039> DWF=[0] (cms), CN/C=[80], TP=[0.97]hrs,
00040> RAINFALL=[ , , , ](mm/hr), END=-1
00041> #-----|
00042> # Dry End-of-Pipe SWM Facility
00043> #-----|
00044> ROUTE RESERVOIR IDout=[5], NHYD=["501"], IDin=[3],
00045> RDT=[1] (min),
00046> TABLE of ( OUTFLOW-STORAGE ) values
00047> (cms) - (ha-m)
00048> [ 0.0 , 0.0 ]
00049> [ 0.001 , 0.0018 ]
00050> [ 0.002 , 0.0104 ]
00051> [ 0.006 , 0.0181 ]
00052> [ 0.014 , 0.0277 ]
00053> [ 0.027 , 0.0387 ]
00054> [ 0.044 , 0.0509 ]
00055> [ 0.073 , 0.0642 ]
00056> [ 0.094 , 0.0788 ]
00057> [ 0.111 , 0.0971 ]
00058> [ 0.125 , 0.1188 ]
00059> [ 0.138 , 0.1411 ]
00060> [ 0.209 , 0.1640 ]
00061> [ 0.336 , 0.1876 ]
00062> [ 0.505 , 0.2117 ]
00063> [ 0.713 , 0.2365 ]
00064> [ 0.961 , 0.2620 ]
00065> [ -1 , -1 ] (max twenty pts)
00066> IDovf=[ ], NHYDovf=[ ]
00067> *#-----|
00068> ADD HYD IDsum=[6], NHYD=["401"], IDs to add=[1+2+4+5]
00069> #-----|
00070> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[2]
00071> * ["si_5.STM"] <--storm filename, one per line for NSTORM time
00072> *#-----|
00073> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[3]
00074> * ["si_10.STM"] <--storm filename, one per line for NSTORM tim
00075> *#-----|
00076> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[4]
00077> * ["si_25.STM"] <--storm filename, one per line for NSTORM tim
00078> *#-----|
00079> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[5]
00080> * ["si_50.STM"] <--storm filename, one per line for NSTORM tim
00081> *#-----|
00082> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[6]
00083> * ["si_100.STM"] <--storm filename, one per line for NSTORM ti
00084> *#-----|
00085> FINISH
00086>
00087>
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00001>-----
00002>-----
00003> SSSSS W W M M H H Y Y M M O O 999 999 -----
00004> S W W M M M H H Y Y M M O O 9 9 9 9
00005> SSSSS W W M M M H H H Y Y M M O O ## 9 9 9 9 Ver. 4.02
00006> S W W M M H H H Y Y M M O O 9999 9999 July 1999
00007> SSSSS W W M M H H Y Y M M O O 9 9 9 9
00008>-----
00009> StormWater Management Hydrologic Model 999 999 -----
00010>-----
00011> *****
00012> ***** SWMHYMO-99 Ver/4.02 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHYMO-83 and OTTHYMO-89. *****
00016>-----
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Ottawa, Ontario: (613) 727-5199 *****
00019> ***** Gatineau, Quebec: (819) 243-6858 *****
00020> ***** E-Mail: swmhyo@jfsa.com *****
00021>-----
00022>-----
00023>-----
00024> ***** Licensed user: Stantec Consulting Ltd. (Kitchener) *****
00025> ***** Kitchener SERIAL#:4730904 *****
00026>-----
00027>-----
00028>-----
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 15000 *****
00032> ***** Max. number of flow points : 15000 *****
00033>-----
00034>-----
00035>-----
00036> ***** DETAILED OUTPUT *****
00037>-----
00038> ***** DATE: 2011-01-31 TIME: 21:52:12 RUN COUNTER: 001067 *****
00039>-----
00040> * Input filename: C:\PROGRA-1\SWMHYMO\Sub_Pro.dat *
00041> * Output filename: C:\PROGRA-1\SWMHYMO\Sub_Pro.out *
00042> * Summary filename: C:\PROGRA-1\SWMHYMO\Sub_Pro.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047>-----
00048>-----
00049>-----
00050> 001:0001-----
00051> # *****
00052> # Project Name : Grand Renewable Energy Park-Substation
00053> # Project Number: 1610-10624
00054> # Date : 1-24-2011
00055> # Company : Stantec Consulting Ltd. (Kitchener)
00056> # Modeller : George Golding, EIT
00057> # Reviewed / Revised : S Robertson (Jan 31, 2011)
00058> # License # : 4730904
00059>-----
00060> # PROPOSED CONDITIONS
00061>-----
00062>-----
00063> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00064> | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00065> | TZERO = .00 hrs on 0
00066> | METOUT= 2 (output = METRIC)
00067> | NRUN = 001
00068> | NSTORM= 1
00069> | # 1=si_2.STM
00070>-----
00071> 001:0002-----
00072>-----
00073> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_2.STM
00074> | Ptotal= 50.70 mm | Comments: SCS-II 24H 2-YEAR SIMCOE
00075>-----
00076> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00077> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00078> .25 .558 | 6.25 1.014 | 12.25 7.301 | 18.25 .913
00079> .50 .558 | 6.50 1.014 | 12.50 7.301 | 18.50 .913
00080> .75 .558 | 6.75 1.014 | 12.75 7.352 | 18.75 .913
00081> 1.00 .558 | 7.00 1.014 | 13.00 3.752 | 19.00 .913
00082> 1.25 .558 | 7.25 1.014 | 13.25 2.636 | 19.25 .913
00083> 1.50 .558 | 7.50 1.014 | 13.50 2.839 | 19.50 .913
00084> 1.75 .558 | 7.75 1.014 | 13.75 2.129 | 19.75 .913
00085> 2.00 .558 | 8.00 1.014 | 14.00 2.129 | 20.00 .913
00086> 2.25 .659 | 8.25 1.369 | 14.25 1.521 | 20.25 .608
00087> 2.50 .659 | 8.50 1.369 | 14.50 1.521 | 20.50 .608
00088> 2.75 .659 | 8.75 1.369 | 14.75 1.521 | 20.75 .608
00089> 3.00 .659 | 9.00 1.369 | 15.00 1.521 | 21.00 .608
00090> 3.25 .659 | 9.25 1.622 | 15.25 1.521 | 21.25 .608
00091> 3.50 .659 | 9.50 1.622 | 15.50 1.521 | 21.50 .608
00092> 3.75 .659 | 9.75 1.825 | 15.75 1.521 | 21.75 .608
00093> 4.00 .659 | 10.00 1.825 | 16.00 1.521 | 22.00 .608
00094> 4.25 .811 | 10.25 2.332 | 16.25 .913 | 22.25 .608
00095> 4.50 .811 | 10.50 2.332 | 16.50 .913 | 22.50 .608
00096> 4.75 .811 | 10.75 3.143 | 16.75 .913 | 22.75 .608
00097> 5.00 .811 | 11.00 3.143 | 17.00 .913 | 23.00 .608
00098> 5.25 .811 | 11.25 4.867 | 17.25 .913 | 23.25 .608
00099> 5.50 .811 | 11.50 4.867 | 17.50 .913 | 23.50 .608
00100> 5.75 .811 | 11.75 21.091 | 17.75 .913 | 23.75 .608
00101> 6.00 .811 | 12.00 55.973 | 18.00 .913 | 24.00 .608
00102>-----
00103>-----
00104> 001:0003-----
00105> # *****
00106> # Catchment North of Haldimand Road
00107>-----
00108>-----
00109> | DESIGN NASHYD | Area (ha)= 10.62 Curve Number (CN)=81.00
00110> | 01:201 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00111> | U.H. Tp(hrs)= 1.080
00112>-----
00113> Unit Hyd Qpeak (cms)= .376
00114>-----
00115> PEAK FLOW (cms)= .176 (i)
00116> TIME TO PEAK (hrs)= 13.083
00117> RUNOFF VOLUME (mm)= 22.252
00118> TOTAL RAINFALL (mm)= 50.699
00119> RUNOFF COEFFICIENT = .439
00120>-----
00121> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00122>-----
00123>-----
00124> 001:0004-----
00125> # *****
00126> # Catchment North of Haldimand Road
00127>-----
00128>-----
00129> | DESIGN NASHYD | Area (ha)= 18.25 Curve Number (CN)=83.00
00130> | 02:202 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00131> | U.H. Tp(hrs)= 1.230
00132>-----
00133> Unit Hyd Qpeak (cms)= .567
00134>-----
00135> PEAK FLOW (cms)= .297 (i)

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00136> TIME TO PEAK (hrs)= 13.250
00137> RUNOFF VOLUME (mm)= 23.913
00138> TOTAL RAINFALL (mm)= 50.699
00139> RUNOFF COEFFICIENT = .472
00140>-----
00141> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00142>-----
00143>-----
00144> 001:0005-----
00145> # *****
00146> # Catchment North of Haldimand Road
00147>-----
00148>-----
00149> | DESIGN STANHYD | Area (ha)= 2.53
00150> | 03:203 DT= 1.00 | Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00
00151>-----
00152> IMPERVIOUS PEROVIOUS (i)
00153> Surface Area (ha)= 1.52 1.01
00154> Dep. Storage (mm)= .80 1.50
00155> Average Slope (%)= .43 .43
00156> Length (m)= 129.87 40.00
00157> Mannings n = .013 .250
00158>-----
00159> Max.eff.Inten.(mm/hr)= 55.97 21.21
00160> over (min) 5.00 26.00
00161> Storage Coeff. (min)= 4.86 (ii) 25.67 (ii)
00162> Unit Hyd. Tpeak (min)= 5.00 26.00
00163> Unit Hyd. peak (cms)= .23 .04
00164>-----
00165> PEAK FLOW (cms)= .22 .04 .244 (iii)
00166> TIME TO PEAK (hrs)= 12.00 12.32 12.000
00167> RUNOFF VOLUME (mm)= 49.90 23.90 39.505
00168> TOTAL RAINFALL (mm)= 50.70 50.70 50.699
00169> RUNOFF COEFFICIENT = .98 .47 .779
00170>-----
00171> (i) CN PROCEDURE SELECTED FOR PEROVIOUS LOSSES:
00172> CN = 83.0 Ia = Dep. Storage (Above)
00173> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00174> THAN THE STORAGE COEFFICIENT.
00175> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00176>-----
00177>-----
00178> 001:0006-----
00179> # *****
00180> # Catchment North of Haldimand Road
00181>-----
00182>-----
00183> | DESIGN NASHYD | Area (ha)= 3.20 Curve Number (CN)=80.00
00184> | 04:204 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00185> | U.H. Tp(hrs)= .970
00186>-----
00187> Unit Hyd Qpeak (cms)= .126
00188>-----
00189> PEAK FLOW (cms)= .055 (i)
00190> TIME TO PEAK (hrs)= 12.917
00191> RUNOFF VOLUME (mm)= 21.478
00192> TOTAL RAINFALL (mm)= 50.699
00193> RUNOFF COEFFICIENT = .424
00194>-----
00195> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00196>-----
00197>-----
00198>-----
00199> # *****
00200> # Dry End-of-Pipe SWM Facility
00201>-----
00202>-----
00203> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00204> | IN<03:203 |
00205> | OUT<05:501 |
00206>-----
00207> OUTFLOW STORAGE TABLE
00208> (cms) (ha.m.) | (cms) (ha.m.)
00209> .000 .000E+00 | .111 .971E+01
00210> .001 .180E+02 | .125 .1188E+02
00211> .002 .104E+01 | .138 .141E+01
00212> .006 .181E+01 | .209 .164E+02
00213> .014 .277E+01 | .336 .187E+02
00214> .027 .387E+01 | .505 .211E+02
00215> .044 .509E+01 | .713 .236E+02
00216> .073 .642E+01 | .961 .220E+02
00217> .094 .788E+01 | .000 .000E+00
00218>-----
00219> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00220> (ha) (cms) (hrs) (mm)
00221> INFLOW>03: (203 ) 2.53 .244 12.000 39.505
00222> OUTFLOW<05: (501 ) 2.53 .043 12.683 39.504
00223>-----
00224> PEAK FLOW REDUCTION [Qout/Qin] (%) = 17.489
00225> TIME SHIFT OF PEAK FLOW (min)= 41.000
00226> MAXIMUM STORAGE USED (ha.m.)=.4998E-01
00227>-----
00228> 001:0008-----
00229>-----
00230> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00231> (ha) (cms) (hrs) (mm) (cms)
00232> ID1 01:201 10.62 .176 13.08 22.25 .000
00233> ID2 02:202 18.25 .297 13.25 23.91 .000
00234> ID3 04:204 3.20 .055 12.92 21.48 .000
00235> ID4 05:501 2.53 .043 12.68 39.50 .000
00236>-----
00237> SUM 06:401 34.60 .568 13.08 24.32 .000
00238>-----
00239> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00240>-----
00241>-----
00242> 001:0009-----
00243> # *****
00244> ** END OF RUN : 1
00245>-----
00246>-----
00247>-----
00248>-----
00249>-----
00250>-----
00251>-----
00252>-----
00253> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00254> | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00255> | TZERO = .00 hrs on 0
00256> | METOUT= 2 (output = METRIC)
00257> | NRUN = 002
00258> | NSTORM= 1
00259> | # 1=si_5.STM
00260>-----
00261> 002:0002-----
00262> # *****
00263> # Project Name : Grand Renewable Energy Park-Substation
00264> # Project Number: 1610-10624
00265> # Date : 1-24-2011
00266> # Company : Stantec Consulting Ltd. (Kitchener)
00267> # Modeller : George Golding, EIT
00268> # Reviewed / Revised : S Robertson (Jan 31, 2011)
00269> # License # : 4730904
00270> # *****

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00271> *# PROPOSED CONDITIONS
00272> *****
00273> -----
00274> 002:0002-----
00275> -----
00276> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_5.STM
00277> | Ptotal= 66.80 mm | Comments: SCS-II 24H 5-YEAR SIMCOE
00278> -----
00279> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00280> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00281> |.25 .735 | 6.25 1.336 | 12.25 2.806 | 18.25 4.943
00282> |.50 .735 | 6.50 1.336 | 12.50 2.806 | 18.50 4.943
00283> |.75 .735 | 6.75 1.336 | 12.75 2.806 | 18.75 4.943
00284> |1.00 .735 | 7.00 1.336 | 13.00 2.806 | 19.00 4.943
00285> |1.25 .735 | 7.25 1.336 | 13.25 2.806 | 19.25 4.943
00286> |1.50 .735 | 7.50 1.336 | 13.50 2.806 | 19.50 4.943
00287> |1.75 .735 | 7.75 1.336 | 13.75 2.806 | 19.75 4.943
00288> |2.00 .735 | 8.00 1.336 | 14.00 2.806 | 20.00 4.943
00289> |2.25 .868 | 8.25 1.804 | 14.25 2.004 | 20.25 .802
00290> |2.50 .868 | 8.50 1.804 | 14.50 2.004 | 20.50 .802
00291> |2.75 .868 | 8.75 1.804 | 14.75 2.004 | 20.75 .802
00292> |3.00 .868 | 9.00 1.804 | 15.00 2.004 | 21.00 .802
00293> |3.25 .868 | 9.25 1.138 | 15.25 2.004 | 21.25 .802
00294> |3.50 .868 | 9.50 1.138 | 15.50 2.004 | 21.50 .802
00295> |3.75 .868 | 9.75 2.405 | 15.75 2.004 | 21.75 .802
00296> |4.00 .868 | 10.00 2.405 | 16.00 2.004 | 22.00 .802
00297> |4.25 1.069 | 10.25 3.073 | 16.25 1.202 | 22.25 .802
00298> |4.50 1.069 | 10.50 3.073 | 16.50 1.202 | 22.50 .802
00299> |4.75 1.069 | 10.75 4.142 | 16.75 1.202 | 22.75 .802
00300> |5.00 1.069 | 11.00 4.142 | 17.00 1.202 | 23.00 .802
00301> |5.25 1.069 | 11.25 6.413 | 17.25 1.202 | 23.25 .802
00302> |5.50 1.069 | 11.50 6.413 | 17.50 1.202 | 23.50 .802
00303> |5.75 1.069 | 11.75 21.789 | 17.75 1.202 | 23.75 .802
00304> |6.00 1.069 | 12.00 73.747 | 18.00 1.202 | 24.00 .802
00305> -----
00306> -----
00307> 002:0003-----
00308> *#-----
00309> *# Catchment North of Haldimand Road
00310> *#-----
00311> -----
00312> | DESIGN NASHYD | Area (ha)= 10.62 Curve Number (CN)=81.00
00313> | 01:201 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00314> | U.H. Tp(hrs)= 1.080
00315> -----
00316> Unit Hyd Qpeak (cms)= .376
00317> -----
00318> PEAK FLOW (cms)= .274 (i)
00319> TIME TO PEAK (hrs)= 13.000
00320> RUNOFF VOLUME (mm)= 34.146
00321> TOTAL RAINFALL (mm)= 66.801
00322> RUNOFF COEFFICIENT = .511
00323> -----
00324> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00325> -----
00326> -----
00327> 002:0004-----
00328> *#-----
00329> *# Catchment North of Haldimand Road
00330> *#-----
00331> -----
00332> | DESIGN NASHYD | Area (ha)= 18.25 Curve Number (CN)=83.00
00333> | 02:202 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00334> | U.H. Tp(hrs)= 1.230
00335> -----
00336> Unit Hyd Qpeak (cms)= .567
00337> -----
00338> PEAK FLOW (cms)= 4.56 (i)
00339> TIME TO PEAK (hrs)= 13.167
00340> RUNOFF VOLUME (mm)= 36.346
00341> TOTAL RAINFALL (mm)= 66.801
00342> RUNOFF COEFFICIENT = .544
00343> -----
00344> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00345> -----
00346> -----
00347> 002:0005-----
00348> *#-----
00349> *# Catchment North of Haldimand Road
00350> *#-----
00351> -----
00352> | DESIGN STANDHYD | Area (ha)= 2.53 Curve Number (CN)=83.00
00353> | 03:203 DT= 1.00 | Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00
00354> | IMPERVIOUS PERVIOUS (i)
00355> | Surface Area (ha)= 1.52 1.01
00356> | Dep. Storage (mm)= .80 1.50
00357> | Average Slope (%)= .43 .43
00358> | Length (m)= 129.87 40.00
00359> | Mannings n = .013 .250
00360> -----
00361> Max. eff. Inten. (mm/hr)= 73.75 37.11
00362> | over (min)= 4.00 21.00
00363> | Storage Coeff. (min)= 4.35 (ii) 20.99 (ii)
00364> | Unit Hyd. Tpeak (min)= 4.00 21.00
00365> | Unit Hyd. peak (cms)= .27 .05
00366> -----
00367> *#-----
00368> PEAK FLOW (cms)= .30 .06 *TOTALS*
00369> | TIME TO PEAK (hrs)= 12.00 12.23 12.000 342 (iii)
00370> | RUNOFF VOLUME (mm)= 65.99 36.33 54.138
00371> | TOTAL RAINFALL (mm)= 66.80 66.80 66.801
00372> | RUNOFF COEFFICIENT = .99 .54 .810
00373> -----
00374> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00375> CN* = 83.0 Ia = Dep. Storage (Above)
00376> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00377> THAN THE STORAGE COEFFICIENT.
00378> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00379> -----
00380> -----
00381> 002:0006-----
00382> *#-----
00383> *# Catchment North of Haldimand Road
00384> *#-----
00385> -----
00386> | DESIGN NASHYD | Area (ha)= 3.20 Curve Number (CN)=80.00
00387> | 04:204 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00388> | U.H. Tp(hrs)= .970
00389> -----
00390> Unit Hyd Qpeak (cms)= .126
00391> -----
00392> PEAK FLOW (cms)= .086 (i)
00393> TIME TO PEAK (hrs)= 12.99
00394> RUNOFF VOLUME (mm)= 33.107
00395> TOTAL RAINFALL (mm)= 66.801
00396> RUNOFF COEFFICIENT = .496
00397> -----
00398> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00399> -----
00400> -----
00401> 002:0007-----
00402> *#-----
00403> *# Dry End-of-Pipe SWM Facility
00404> *#-----
00405> -----

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00406> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00407> | IN>03: (203 ) |
00408> | OUT<05: (501 ) | ----- OUTFLOW STORAGE TABLE -----
00409> | OUTFLOW STORAGE | OUTFLOW STORAGE
00410> | (cms) (ha.m.) | (cms) (ha.m.)
00411> |.000 .0000E+00 | .111 .9710E-01
00412> |.001 .1800E-02 | .125 .1188E+00
00413> |.002 .1040E-01 | .138 .1411E+00
00414> |.006 .1810E-01 | .209 .1640E+00
00415> |.014 .2770E-01 | .336 .1876E+00
00416> |.027 .3870E-01 | .505 .2117E+00
00417> |.044 .5090E-01 | .713 .2365E+00
00418> |.073 .6420E-01 | .961 .2620E+00
00419> |.094 .7880E-01 | .000 .0000E+00
00420> -----
00421> ROUTING RESULTS AREA APEAK TPEAK R.V.
00422> | (ha) (cms) (hrs) (mm)
00423> | INFLOW >03: (203 ) 2.53 .342 12.000 54.138
00424> | OUTFLOW<05: (501 ) 2.53 .074 12.567 54.138
00425> -----
00426> PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.720
00427> TIME SHIFT OF PEAK FLOW (min)= 34.00
00428> MAXIMUM STORAGE USED (ha.m.)= .6510E-01
00429> -----
00430> -----
00431> 002:0008-----
00432> *#-----
00433> | ADD HXD (401 ) | ID: NHYD AREA APEAK TPEAK R.V. DWF
00434> | (ha) (cms) (hrs) (mm) (cms)
00435> | ID1 01:201 10.62 .274 13.00 34.15 .000
00436> | ID2 02:202 18.25 .456 13.17 36.35 .000
00437> | ID3 04:204 3.20 .086 12.92 33.11 .000
00438> | ID4 05:501 2.53 .074 12.57 54.14 .000
00439> | NSTORM= 1
00440> | SUM 06:401 34.60 .878 13.08 36.67 .000
00441> -----
00442> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00443> -----
00444> -----
00445> 002:0009-----
00446> *#-----
00447> -----
00448> -----
00449> ** END OF RUN : 2
00450> -----
00451> *****
00452> -----
00453> -----
00454> -----
00455> -----
00456> -----
00457> -----
00458> | START | Project dir.: C:\PROGRA-1\SWMHYMO
00459> | Rainfall dir.: C:\PROGRA-1\SWMHYMO
00460> | TZERO = .00 hrs on 0
00461> | METOUT= 2 (output = METRIC)
00462> | NRUN = 003
00463> | NSTORM= 1
00464> | # l=si_10.STM
00465> -----
00466> 003:0002-----
00467> *****
00468> *# Project Name : Grand Renewable Energy Park-Substation
00469> *# Project Number: 1610-10624
00470> *# Date : 1-24-2011
00471> *# Company : Stantec Consulting Ltd. (Kitchener)
00472> *# Modeller : George Golding, EIT
00473> *# Reviewed / Revised : S Robertson (Jan 31, 2011)
00474> *# License # : 4730904
00475> *****
00476> *# PROPOSED CONDITIONS
00477> *****
00478> -----
00479> 003:0002-----
00480> -----
00481> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_10.STM
00482> | Ptotal= 77.40 mm | Comments: SCS-II 24H 10-YEAR SIMCOE
00483> -----
00484> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00485> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00486> |.25 .851 | 6.25 1.548 | 12.25 11.146 | 18.25 1.393
00487> |.50 .851 | 6.50 1.548 | 12.50 11.146 | 18.50 1.393
00488> |.75 .851 | 6.75 1.548 | 12.75 5.728 | 18.75 1.393
00489> |1.00 .851 | 7.00 1.548 | 13.00 5.728 | 19.00 1.393
00490> |1.25 .851 | 7.25 1.548 | 13.25 4.025 | 19.25 1.393
00491> |1.50 .851 | 7.50 1.548 | 13.50 4.334 | 19.50 1.393
00492> |1.75 .851 | 7.75 1.548 | 13.75 3.251 | 19.75 1.393
00493> |2.00 .851 | 8.00 1.548 | 14.00 3.251 | 20.00 1.393
00494> |2.25 1.006 | 8.25 2.090 | 14.25 2.322 | 20.25 .929
00495> |2.50 1.006 | 8.50 2.090 | 14.50 2.322 | 20.50 .929
00496> |2.75 1.006 | 8.75 2.090 | 14.75 2.322 | 20.75 .929
00497> |3.00 1.006 | 9.00 2.090 | 15.00 2.322 | 21.00 .929
00498> |3.25 1.006 | 9.25 2.477 | 15.25 2.322 | 21.25 .929
00499> |3.50 1.006 | 9.50 2.477 | 15.50 2.322 | 21.50 .929
00500> |3.75 1.006 | 9.75 2.786 | 15.75 2.322 | 21.75 .929
00501> |4.00 1.006 | 10.00 2.786 | 16.00 2.322 | 22.00 .929
00502> |4.25 1.238 | 10.25 3.560 | 16.25 2.393 | 22.25 .929
00503> |4.50 1.238 | 10.50 3.560 | 16.50 1.393 | 22.50 .929
00504> |4.75 1.238 | 10.75 4.799 | 16.75 1.393 | 22.75 .929
00505> |5.00 1.238 | 11.00 4.799 | 17.00 1.393 | 23.00 .929
00506> |5.25 1.238 | 11.25 7.430 | 17.25 1.393 | 23.25 .929
00507> |5.50 1.238 | 11.50 7.430 | 17.50 1.393 | 23.50 .929
00508> |5.75 1.238 | 11.75 32.198 | 17.75 1.393 | 23.75 .929
00509> |6.00 1.238 | 12.00 85.450 | 18.00 1.393 | 24.00 .929
00510> -----
00511> -----
00512> 003:0003-----
00513> *#-----
00514> *# Catchment North of Haldimand Road
00515> *#-----
00516> -----
00517> | DESIGN NASHYD | Area (ha)= 10.62 Curve Number (CN)=81.00
00518> | 01:201 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00519> | U.H. Tp(hrs)= 1.080
00520> -----
00521> Unit Hyd Qpeak (cms)= .376
00522> -----
00523> PEAK FLOW (cms)= .343 (i)
00524> TIME TO PEAK (hrs)= 13.000
00525> RUNOFF VOLUME (mm)= 42.520
00526> TOTAL RAINFALL (mm)= 77.398
00527> RUNOFF COEFFICIENT = .549
00528> -----
00529> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00530> -----
00531> -----
00532> 003:0004-----
00533> *#-----
00534> *# Catchment North of Haldimand Road
00535> *#-----
00536> -----
00537> | DESIGN NASHYD | Area (ha)= 18.25 Curve Number (CN)=83.00
00538> | 02:202 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00539> | U.H. Tp(hrs)= 1.230
00540> -----

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00541> Unit Hyd Qpeak (cms)= .567
00542>
00543> PEAK FLOW (cms)= .568 (i)
00544> TIME TO PEAK (hrs)= 13.167
00545> RUNOFF VOLUME (mm)= 45.032
00546> TOTAL RAINFALL (mm)= 77.398
00547> RUNOFF COEFFICIENT = .582
00548>
00549> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00550>
00551> -----
00552> 003:0005-----
00553> *#-----
00554> *# Catchment North of Haldimand Road
00555> *#-----
00556> -----
00557> | DESIGN STANDHYD | Area (ha)= 2.53
00558> | 03:203 DT= 1.00 | Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00
00559> -----
00560> IMPERVIOUS PERVIOUS (i)
00561> Surface Area (ha)= 1.52 1.01
00562> Dep. Storage (mm)= .80 1.50
00563> Average Slope (%)= .43 .43
00564> Length (m)= 129.87 40.00
00565> Mannings n = .013 .250
00566>
00567> Max. eff. Inten. (mm/hr)= 85.45 49.05
00568> over (min)= 4.00 19.00
00569> Storage Coeff. (min)= 4.10 (ii) 18.98 (ii)
00570> Unit Hyd. Tpeak (min)= 4.00 19.00
00571> Unit Hyd. peak (cms)= .28 .06
00572>
00573> PEAK FLOW (cms)= .35 .09 *TOTALS*
00574> TIME TO PEAK (hrs)= 12.00 12.20 12.000 (iii)
00575> RUNOFF VOLUME (mm)= 76.59 45.02 63.972
00576> TOTAL RAINFALL (mm)= 77.40 77.40 77.399
00577> RUNOFF COEFFICIENT = .99 .58 .827
00578>
00579> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00580> CN* = 83.0 Ia = Dep. Storage (Above)
00581> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00582> THAN THE STORAGE COEFFICIENT.
00583> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00584>
00585> -----
00586> 003:0006-----
00587> *#-----
00588> *# Catchment North of Haldimand Road
00589> *#-----
00590> -----
00591> | DESIGN NASHYD | Area (ha)= 3.20 Curve Number (CN)=80.00
00592> | 04:204 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00593> | U.H. Tp(hrs)= .970
00594>
00595> Unit Hyd Qpeak (cms)= .126
00596>
00597> PEAK FLOW (cms)= .108 (i)
00598> TIME TO PEAK (hrs)= 12.917
00599> RUNOFF VOLUME (mm)= 41.324
00600> TOTAL RAINFALL (mm)= 77.398
00601> RUNOFF COEFFICIENT = .534
00602>
00603> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00604>
00605> -----
00606> 003:0007-----
00607> *#-----
00608> *# Dry End-of-Pipe SWM Facility
00609> *#-----
00610> -----
00611> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00612> | IN>03:(203 ) |
00613> | OUT<05:(501 ) |
00614> -----
00615> | OUTFLOW STORAGE | OUTFLOW STORAGE |
00616> | (cms) (ha.m.) | (cms) (ha.m.) |
00617> | .000 .0000E+00 | .111 .9710E-01 |
00618> | .001 .1800E-02 | .125 .1188E+00 |
00619> | .002 .1040E-01 | .138 .1411E+00 |
00620> | .006 .1810E-01 | .209 .1640E+00 |
00621> | .014 .2770E-01 | .336 .1876E+00 |
00622> | .027 .3870E-01 | .505 .2117E+00 |
00623> | .044 .5090E-01 | .713 .2365E+00 |
00624> | .073 .6420E-01 | .961 .2620E+00 |
00625> | .094 .7880E-01 | .000 .0000E+00 |
00626>
00627> ROUTING RESULTS AREA OPEAK TPEAK R.V.
00628> (ha) (cms) (hrs) (mm)
00629> INFLOW<03: (203 ) 2.53 .409 12.000 63.972
00630> OUTFLOW<05: (501 ) 2.53 .090 12.550 63.972
00631>
00632> PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.931
00633> TIME SHIFT OF PEAK FLOW (min) = 33.00
00634> MAXIMUM STORAGE USED (ha.m.) = .7580E-01
00635>
00636> 003:0008-----
00637> *#-----
00638> | ADD HYD (401 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00639> (ha) (cms) (hrs) (mm) (cms)
00640> +ID1 01:201 10.62 .343 13.00 42.52 .000
00641> +ID2 02:202 18.25 .568 13.17 45.03 .000
00642> +ID3 04:204 3.20 .108 12.92 41.32 .000
00643> +ID4 05:501 2.53 .090 12.55 63.97 .000
00644>
00645> SUM 06:401 34.60 1.095 13.08 45.30 .000
00646>
00647> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00648>
00649> -----
00650> 003:0009-----
00651> *#-----
00652> *#-----
00653> 003:0002-----
00654> *#-----
00655> 003:0002-----
00656> ** END OF RUN : 3
00657>
00658> *****
00659>
00660>
00661>
00662>
00663>
00664>
00665> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00666> | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00667> | TZERO = .00 hrs on 0
00668> | METOUT = 2 (output = METRIC)
00669> | NRUN = 004
00670> | NSTORM = 1
00671> | # 1=si_25.STM
00672>
00673> 004:0002-----
00674> *#-----
00675> *# Project Name : Grand Renewable Energy Park-Substation

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00676> *# Project Number: 1610-10624
00677> *# Date : 1-24-2011
00678> *# Company : Stantec Consulting Ltd. (Kitchener)
00679> *# Modeller : George Golding, EIT
00680> *# Reviewed / Revised : S Robertson (Jan 31, 2011)
00681> *# License # : 4730904
00682> *#*****
00683> *# PROPOSED CONDITIONS
00684> *#*****
00685>
00686> 004:0002-----
00687> *#-----
00688> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_25.STM
00689> | Ptotal= 90.80 mm | Comments: SCS-II 24H 25-YEAR SIMCOE
00690>
00691> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00692> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00693> .25 .999 | 6.25 1.816 | 12.25 13.075 | 18.25 1.634
00694> .50 .999 | 6.50 1.816 | 12.50 13.075 | 18.50 1.634
00695> .75 .999 | 6.75 1.816 | 12.75 6.719 | 18.75 1.634
00696> 1.00 .999 | 7.00 1.816 | 13.00 6.719 | 19.00 1.634
00697> 1.25 .999 | 7.25 1.816 | 13.25 4.722 | 19.25 1.634
00698> 1.50 .999 | 7.50 1.816 | 13.50 3.085 | 19.50 1.634
00699> 1.75 .999 | 7.75 1.816 | 13.75 3.814 | 19.75 1.634
00700> 2.00 .999 | 8.00 1.816 | 14.00 3.814 | 20.00 1.634
00701> 2.25 1.180 | 8.25 2.452 | 14.25 2.724 | 20.25 1.090
00702> 2.50 1.180 | 8.50 2.452 | 14.50 2.724 | 20.50 1.090
00703> 2.75 1.180 | 8.75 2.452 | 14.75 2.724 | 20.75 1.090
00704> 3.00 1.180 | 9.00 2.452 | 15.00 2.724 | 21.00 1.090
00705> 3.25 1.180 | 9.25 2.906 | 15.25 2.724 | 21.25 1.090
00706> 3.50 1.180 | 9.50 2.906 | 15.50 2.724 | 21.50 1.090
00707> 3.75 1.180 | 9.75 3.269 | 15.75 2.724 | 21.75 1.090
00708> 4.00 1.180 | 10.00 3.269 | 16.00 2.724 | 22.00 1.090
00709> 4.25 1.453 | 10.25 4.177 | 16.25 1.634 | 22.25 1.090
00710> 4.50 1.453 | 10.50 4.177 | 16.50 1.634 | 22.50 1.090
00711> 4.75 1.453 | 10.75 5.630 | 16.75 1.634 | 22.75 1.090
00712> 5.00 1.453 | 11.00 5.630 | 17.00 1.634 | 23.00 1.090
00713> 5.25 1.453 | 11.25 8.717 | 17.25 1.634 | 23.25 1.090
00714> 5.50 1.453 | 11.50 8.717 | 17.50 1.634 | 23.50 1.090
00715> 5.75 1.453 | 11.75 37.773 | 17.75 1.634 | 23.75 1.090
00716> 6.00 1.453 | 12.00 100.243 | 18.00 1.634 | 24.00 1.090
00717>
00718>
00719> 004:0003-----
00720> *#-----
00721> *# Catchment North of Haldimand Road
00722> *#-----
00723> -----
00724> | DESIGN NASHYD | Area (ha)= 10.62 Curve Number (CN)=81.00
00725> | 01:201 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00726> | U.H. Tp(hrs)= 1.080
00727>
00728> Unit Hyd Qpeak (cms)= .376
00729>
00730> PEAK FLOW (cms)= .434 (i)
00731> TIME TO PEAK (hrs)= 13.000
00732> RUNOFF VOLUME (mm)= 53.564
00733> TOTAL RAINFALL (mm)= 90.801
00734> RUNOFF COEFFICIENT = .590
00735>
00736> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00737>
00738> -----
00739> 004:0004-----
00740> *#-----
00741> *# Catchment North of Haldimand Road
00742> *#-----
00743> -----
00744> | DESIGN NASHYD | Area (ha)= 18.25 Curve Number (CN)=83.00
00745> | 02:202 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00746> | U.H. Tp(hrs)= 1.230
00747>
00748> Unit Hyd Qpeak (cms)= .567
00749>
00750> PEAK FLOW (cms)= .715 (i)
00751> TIME TO PEAK (hrs)= 13.167
00752> RUNOFF VOLUME (mm)= 56.428
00753> TOTAL RAINFALL (mm)= 90.801
00754> RUNOFF COEFFICIENT = .621
00755>
00756> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00757>
00758> -----
00759> 004:0005-----
00760> *#-----
00761> *# Catchment North of Haldimand Road
00762> *#-----
00763> -----
00764> | DESIGN STANDHYD | Area (ha)= 2.53
00765> | 03:203 DT= 1.00 | Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00
00766>
00767> Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
00768> Dep. Storage (mm)= .80 1.50
00769> Average Slope (%)= .43 .43
00770> Length (m)= 129.87 40.00
00771> Mannings n = .013 .250
00772>
00773> Max. eff. Inten. (mm/hr)= 100.24 65.99
00774> over (min)= 4.00 17.00
00775> Storage Coeff. (min)= 3.85 (ii) 17.06 (ii)
00776> Unit Hyd. Tpeak (min)= 4.00 17.00
00777> Unit Hyd. peak (cms)= .29 .07
00778>
00779>
00780> PEAK FLOW (cms)= .41 .12 *TOTALS*
00781> TIME TO PEAK (hrs)= 12.00 12.17 12.000
00782> RUNOFF VOLUME (mm)= 89.99 56.41 76.572
00783> TOTAL RAINFALL (mm)= 90.80 90.80 90.802
00784> RUNOFF COEFFICIENT = .99 .62 .843
00785>
00786> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00787> CN* = 83.0 Ia = Dep. Storage (Above)
00788> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00789> THAN THE STORAGE COEFFICIENT.
00790> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00791>
00792> -----
00793> 004:0006-----
00794> *#-----
00795> *# Catchment North of Haldimand Road
00796> *#-----
00797> -----
00798> | DESIGN NASHYD | Area (ha)= 3.20 Curve Number (CN)=80.00
00799> | 04:204 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00800> | U.H. Tp(hrs)= .970
00801>
00802> Unit Hyd Qpeak (cms)= .126
00803>
00804> PEAK FLOW (cms)= .138 (i)
00805> TIME TO PEAK (hrs)= 12.917
00806> RUNOFF VOLUME (mm)= 52.190
00807> TOTAL RAINFALL (mm)= 90.801
00808> RUNOFF COEFFICIENT = .575
00809>
00810> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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00811>
00812>
00813> 004:0007
00814> *#-----
00815> *# Dry End-of-Pipe SWM Facility
00816> *#-----
00817>
00818> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00819> | IN>03:(203 ) |
00820> | OUT<05:(501 ) |
00821> |----- OUTFLOW STORAGE TABLE -----|
00822> | OUTFLOW STORAGE | OUTFLOW STORAGE |
00823> | (cms) (ha.m.) | (cms) (ha.m.) |
00824> | .000 .000E+00 | .111 .9710E-01 |
00825> | .001 .1800E-02 | .125 .1188E+00 |
00826> | .002 .1040E-01 | .138 .1411E+00 |
00827> | .006 .1810E-01 | .209 .1640E+00 |
00828> | .014 .2770E-01 | .336 .1876E+00 |
00829> | .027 .3870E-01 | .505 .2117E+00 |
00830> | .044 .5090E-01 | .713 .2365E+00 |
00831> | .073 .6420E-01 | .961 .2620E+00 |
00832> | .094 .7880E-01 | .000 .0000E+00 |
00833>
00834> ROUTING RESULTS AREA QPEAK TPEAK R.V.
00835> (ha) (cms) (hrs) (mm)
00836> INFLOW>03: (203 ) 2.53 .498 12.000 76.572
00837> OUTFLOW<05: (501 ) 2.53 .105 12.550 76.572
00838>
00839> PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.025
00840> TIME SHIFT OF PEAK FLOW (min) = 33.00
00841> MAXIMUM STORAGE USED (ha.m.) = .9028E-01
00842>
00843> 004:0008
00844> *#-----
00845> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00846> | (ha) (cms) (hrs) (mm) (cms) |
00847> | ID1 01:201 | 10.62 .434 13.00 53.56 .000
00848> | +ID2 02:202 | 18.25 .715 13.17 56.43 .000
00849> | +ID3 04:204 | 3.20 .138 12.92 52.19 .000
00850> | +ID4 05:501 | 2.53 .105 12.55 76.57 .000
00851>
00852> |-----
00853> | SUM 06:401 | 34.60 1.377 13.08 56.63 .000
00854>
00855> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00856>
00857> 004:0009
00858> *#-----
00859>
00860> 004:0002
00861>
00862> 004:0002
00863>
00864> 004:0002
00865> ** END OF RUN : 4
00866>
00867> *****
00868>
00869>
00870>
00871>
00872>
00873>
00874> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00875> | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00876> | TZERO = .00 hrs on 0
00877> | METOUT = 2 (output = METRIC)
00878> | NRUN = 005
00879> | NSTORM = 1
00880> | # 1=si_50.STM
00881>
00882> 005:0002
00883> *#-----
00884> *# Project Name : Grand Renewable Energy Park-Substation
00885> *# Project Number: 1610-10624
00886> *# Date : 1-24-2011
00887> *# Company : Stantec Consulting Ltd. (Kitchener)
00888> *# Modelled by : George Golding, EIT
00889> *# Reviewed / Revised : S Robertson (Jan 31, 2011)
00890> *# License # : 4730904
00891> *****
00892> *# PROPOSED CONDITIONS
00893> *#-----
00894>
00895> 005:0002
00896>
00897> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_50.STM
00898> | Ptotal= 100.70 mm | Comments: SCS-II 24H 50-YEAR SIMCOE
00899>
00900> |-----
00901> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN |
00902> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr |
00903> | .25 1.108 | 6.25 2.014 | 12.25 14.501 | 18.25 1.813 |
00904> | .50 1.108 | 6.50 2.014 | 12.50 14.501 | 18.50 1.813 |
00905> | .75 1.108 | 6.75 2.014 | 12.75 14.501 | 18.75 1.813 |
00906> | 1.00 1.108 | 7.00 2.014 | 13.00 14.501 | 19.00 1.813 |
00907> | 1.25 1.108 | 7.25 2.014 | 13.25 14.501 | 19.25 1.813 |
00908> | 1.50 1.108 | 7.50 2.014 | 13.50 14.501 | 19.50 1.813 |
00909> | 1.75 1.108 | 7.75 2.014 | 13.75 14.501 | 19.75 1.813 |
00910> | 2.00 1.108 | 8.00 2.014 | 14.00 14.501 | 20.00 1.813 |
00911> | 2.25 1.309 | 8.25 2.719 | 14.25 3.021 | 20.25 1.208 |
00912> | 2.50 1.309 | 8.50 2.719 | 14.50 3.021 | 20.50 1.208 |
00913> | 2.75 1.309 | 8.75 2.719 | 14.75 3.021 | 20.75 1.208 |
00914> | 3.00 1.309 | 9.00 2.719 | 15.00 3.021 | 21.00 1.208 |
00915> | 3.25 1.309 | 9.25 3.222 | 15.25 3.021 | 21.25 1.208 |
00916> | 3.50 1.309 | 9.50 3.222 | 15.50 3.021 | 21.50 1.208 |
00917> | 3.75 1.309 | 9.75 3.625 | 15.75 3.021 | 21.75 1.208 |
00918> | 4.00 1.309 | 10.00 3.625 | 16.00 3.021 | 22.00 1.208 |
00919> | 4.25 1.611 | 10.25 4.632 | 16.25 1.813 | 22.25 1.208 |
00920> | 4.50 1.611 | 10.50 4.632 | 16.50 1.813 | 22.50 1.208 |
00921> | 4.75 1.611 | 10.75 6.243 | 16.75 1.813 | 22.75 1.208 |
00922> | 5.00 1.611 | 11.00 6.243 | 17.00 1.813 | 23.00 1.208 |
00923> | 5.25 1.611 | 11.25 9.667 | 17.25 1.813 | 23.25 1.208 |
00924> | 5.50 1.611 | 11.50 9.667 | 17.50 1.813 | 23.50 1.208 |
00925> | 5.75 1.611 | 11.75 11.173 | 17.75 1.813 | 23.75 1.208 |
00926> | 6.00 1.611 | 12.00 11.173 | 18.00 1.813 | 24.00 1.208 |
00927>
00928> 005:0003
00929> *#-----
00930> *# Catchment North of Haldimand Road
00931> *#-----
00932>
00933> | DESIGN NASHYD | Area (ha)= 10.62 Curve Number (CN)=81.00
00934> | ID1:201 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00935> | U.H. Tp(hrs)= 1.080
00936>
00937> Unit Hyd Qpeak (cms)= .376
00938>
00939> PEAK FLOW (cms)= .503 (i)
00940> TIME TO PEAK (hrs)= 13.000
00941> RUNOFF VOLUME (mm)= 61.976
00942> TOTAL RAINFALL (mm)= 100.699
00943> RUNOFF COEFFICIENT = .615
00944>
00945> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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00946>
00947> 005:0004
00948> *#-----
00949> *# Catchment North of Haldimand Road
00950> *#-----
00951>
00952> | DESIGN NASHYD | Area (ha)= 18.25 Curve Number (CN)=83.00
00953> | ID1:202 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00954> | U.H. Tp(hrs)= 1.230
00955>
00956> Unit Hyd Qpeak (cms)= .567
00957>
00958> PEAK FLOW (cms)= .826 (i)
00959> TIME TO PEAK (hrs)= 13.167
00960> RUNOFF VOLUME (mm)= 65.073
00961> TOTAL RAINFALL (mm)= 100.699
00962> RUNOFF COEFFICIENT = .646
00963>
00964> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00965>
00966>
00967>
00968> 005:0005
00969> *#-----
00970> *# Catchment North of Haldimand Road
00971> *#-----
00972>
00973> | DESIGN STANHYD | Area (ha)= 2.53
00974> | ID1:203 DT= 1.00 | Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00
00975>
00976> IMPERVIOUS PERVIOUS (i)
00977> Surface Area (ha)= 1.52 1.01
00978> Dep. Storage (mm)= .80 1.50
00979> Average Slope (%)= .43 .43
00980> Length (m)= 129.87 40.00
00981> Mannings n = .013 .250
00982>
00983> Max. eff. Inten. (mm/hr)= 111.17 79.08
00984> over (min) = 4.00 16.00
00985> Storage Coeff. (min)= 3.69 (ii) 15.98 (ii)
00986> Unit Hyd. Tpeak (min)= 4.00 16.00
00987> Unit Hyd. peak (cms)= .30 .07
00988>
00989> PEAK FLOW (cms)= .46 .14 *TOTALS*
00990> TIME TO PEAK (hrs)= 12.00 12.15 12.000
00991> RUNOFF VOLUME (mm)= 99.89 65.05 85.969
00992> TOTAL RAINFALL (mm)= 100.70 100.70 100.700
00993> RUNOFF COEFFICIENT = .99 .65 .854
00994>
00995> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00996> CN* = 83.0 Ia = Dep. Storage (Above)
00997> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00998> THAN THE STORAGE COEFFICIENT.
00999> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01000>
01001>
01002> 005:0006
01003> *#-----
01004> *# Catchment North of Haldimand Road
01005> *#-----
01006>
01007> | DESIGN NASHYD | Area (ha)= 3.20 Curve Number (CN)=80.00
01008> | ID1:204 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01009> | U.H. Tp(hrs)= .970
01010>
01011> Unit Hyd Qpeak (cms)= .126
01012>
01013> PEAK FLOW (cms)= .160 (i)
01014> TIME TO PEAK (hrs)= 12.917
01015> RUNOFF VOLUME (mm)= 60.482
01016> TOTAL RAINFALL (mm)= 100.699
01017> RUNOFF COEFFICIENT = .601
01018>
01019> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01020>
01021>
01022> 005:0007
01023> *#-----
01024> *# Dry End-of-Pipe SWM Facility
01025> *#-----
01026>
01027> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01028> | IN>03:(203 ) |
01029> | OUT<05:(501 ) |
01030> |----- OUTFLOW STORAGE TABLE -----|
01031> | OUTFLOW STORAGE | OUTFLOW STORAGE |
01032> | (cms) (ha.m.) | (cms) (ha.m.) |
01033> | .000 .000E+00 | .111 .9710E-01 |
01034> | .001 .1800E-02 | .125 .1188E+00 |
01035> | .002 .1040E-01 | .138 .1411E+00 |
01036> | .006 .1810E-01 | .209 .1640E+00 |
01037> | .014 .2770E-01 | .336 .1876E+00 |
01038> | .027 .3870E-01 | .505 .2117E+00 |
01039> | .044 .5090E-01 | .713 .2365E+00 |
01040> | .073 .6420E-01 | .961 .2620E+00 |
01041> | .094 .7880E-01 | .000 .0000E+00 |
01042>
01043> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01044> (ha) (cms) (hrs) (mm)
01045> INFLOW>03: (203 ) 2.53 .565 12.000 85.969
01046> OUTFLOW<05: (501 ) 2.53 .114 12.550 85.968
01047>
01048> PEAK FLOW REDUCTION [Qout/Qin] (%) = 20.128
01049> TIME SHIFT OF PEAK FLOW (min) = 33.00
01050> MAXIMUM STORAGE USED (ha.m.) = .1014E+00
01051>
01052> 005:0008
01053> *#-----
01054> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01055> | (ha) (cms) (hrs) (mm) (cms) |
01056> | ID1 01:201 | 10.62 .503 13.00 61.98 .000
01057> | +ID2 02:202 | 18.25 .826 13.17 65.07 .000
01058> | +ID3 04:204 | 3.20 .138 12.92 60.48 .000
01059> | +ID4 05:501 | 2.53 .114 12.55 85.97 .000
01060>
01061> |-----
01062> | SUM 06:401 | 34.60 1.589 13.08 65.23 .000
01063>
01064> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01065>
01066> 005:0009
01067> *#-----
01068>
01069> 005:0002
01070>
01071> 005:0002
01072>
01073> 005:0002
01074>
01075> 005:0002
01076> ** END OF RUN : 5
01077>
01078> *****
01079>
01080>

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01081>
01082>
01083>
01084>
01085> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
01086> |-----| Rainfall dir.: C:\PROGRA-1\SWMHYMO\
01087> | TZERO = .00 hrs on 0
01088> | METOUT= 2 (output = METRIC)
01089> | NRUN = 006
01090> | NSTORM= 1
01091> | # 1=si_100.STM
01092>
01093> 006:0002-----
01094> *****
01095> *# Project Name : Grand Renewable Energy Park-Substation
01096> *# Project Number: 1610-10624
01097> *# Date : 1-24-2011
01098> *# Company : Stantec Consulting Ltd. (Kitchener)
01099> *# Modeller : George Golding, EIT
01100> *# Reviewed / Revised : S Robertson (Jan 31, 2011)
01101> *# License # : 4730904
01102>
01103> *# PROPOSED CONDITIONS
01104> *****
01105>
01106> 006:0002-----
01107>
01108> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_100.STM
01109> | Ptotal= 110.60 mm | Comments: SCS-II 24H 100-YEAR SIMCOE
01110>
01111> |-----|
01112> | TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN |
01113> | hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr |
01114> |.25 1.217 | 6.25 2.212 | 12.25 5.926 | 18.25 1.991 |
01115> |.50 1.217 | 6.50 2.212 | 12.50 15.926 | 18.50 1.991 |
01116> |.75 1.217 | 6.75 2.212 | 12.75 8.184 | 18.75 1.991 |
01117> |1.00 1.217 | 7.00 2.212 | 13.00 8.184 | 19.00 1.991 |
01118> |1.25 1.217 | 7.25 2.212 | 13.25 5.751 | 19.25 1.991 |
01119> |1.50 1.217 | 7.50 2.212 | 13.50 6.194 | 19.50 1.991 |
01120> |1.75 1.217 | 7.75 2.212 | 13.75 4.645 | 19.75 1.991 |
01121> |2.00 1.217 | 8.00 2.212 | 14.00 4.645 | 20.00 1.991 |
01122> |2.25 1.438 | 8.25 2.986 | 14.25 3.318 | 20.25 1.327 |
01123> |2.50 1.438 | 8.50 2.986 | 14.50 3.318 | 20.50 1.327 |
01124> |2.75 1.438 | 8.75 2.986 | 14.75 3.318 | 20.75 1.327 |
01125> |3.00 1.438 | 9.00 2.986 | 15.00 3.318 | 21.00 1.327 |
01126> |3.25 1.438 | 9.25 3.539 | 15.25 3.318 | 21.25 1.327 |
01127> |3.50 1.438 | 9.50 3.539 | 15.50 3.318 | 21.50 1.327 |
01128> |3.75 1.438 | 9.75 3.982 | 15.75 3.318 | 21.75 1.327 |
01129> |4.00 1.438 | 10.00 3.982 | 16.00 3.318 | 22.00 1.327 |
01130> |4.25 1.770 | 10.25 5.088 | 16.25 1.991 | 22.25 1.327 |
01131> |4.50 1.770 | 10.50 5.088 | 16.50 1.991 | 22.50 1.327 |
01132> |4.75 1.770 | 10.75 6.857 | 16.75 1.991 | 22.75 1.327 |
01133> |5.00 1.770 | 11.00 6.857 | 17.00 1.991 | 23.00 1.327 |
01134> |5.25 1.770 | 11.25 10.618 | 17.25 1.991 | 23.25 1.327 |
01135> |5.50 1.770 | 11.50 10.618 | 17.50 1.991 | 23.50 1.327 |
01136> |5.75 1.770 | 11.75 46.010 | 17.75 1.991 | 23.75 1.327 |
01137> |6.00 1.770 | 12.00 122.102 | 18.00 1.991 | 24.00 1.327 |
01138>
01139> 006:0003-----
01140> *#-----|
01141> *# Catchment North of Haldimand Road
01142>
01143>
01144> | DESIGN NASHYD | Area (ha)= 10.62 Curve Number (CN)=81.00
01145> | 01:201 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01146> | U.H. Tp(hrs)= 1.080
01147>
01148> | Unit Hyd Qpeak (cms)= .376
01149>
01150> | PEAK FLOW (cms)= .574 (i)
01151> | TIME TO PEAK (hrs)= 13.000
01152> | RUNOFF VOLUME (mm)= 70.566
01153> | TOTAL RAINFALL (mm)= 110.602
01154> | RUNOFF COEFFICIENT = .638
01155>
01156> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01157>
01158>
01159> 006:0004-----
01160> *#-----|
01161> *# Catchment North of Haldimand Road
01162>
01163>
01164> | DESIGN NASHYD | Area (ha)= 18.25 Curve Number (CN)=83.00
01165> | 02:202 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01166> | U.H. Tp(hrs)= 1.230
01167>
01168> | Unit Hyd Qpeak (cms)= .567
01169>
01170> | PEAK FLOW (cms)= .939 (i)
01171> | TIME TO PEAK (hrs)= 13.167
01172> | RUNOFF VOLUME (mm)= 73.875
01173> | TOTAL RAINFALL (mm)= 110.602
01174> | RUNOFF COEFFICIENT = .668
01175>
01176> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01177>
01178>
01179> 006:0005-----
01180> *#-----|
01181> *# Catchment North of Haldimand Road
01182>
01183>
01184> | DESIGN STANDHYD | Area (ha)= 2.53
01185> | 03:203 DT= 1.00 | Total Imp(%)= 60.00 Dir. Conn.(%)= 60.00
01186>
01187> | IMPERVIOUS PERVIOUS (i)
01188> | Surface Area (ha)= 1.52 1.01
01189> | Dep. Storage (mm)= .80 1.50
01190> | Average Slope (%)= .43 .43
01191> | Length (m)= 129.87 40.00
01192> | Mannings n = .013 .250
01193>
01194> | Max.eff.Inten.(mm/hr)= 122.10 93.51
01195> | over (min)= 4.00 15.00
01196> | Storage Coeff. (min)= 3.55 (ii) 15.05 (ii)
01197> | Unit Hyd. Tpeak (min)= 4.00 15.00
01198> | Unit Hyd. peak (cms)= .30 .08
01199>
01200> | PEAK FLOW (cms)= .51 .16 .636 (iii)
01201> | TIME TO PEAK (hrs)= 12.00 12.13 12.000
01202> | RUNOFF VOLUME (mm)= 109.79 73.86 95.432
01203> | TOTAL RAINFALL (mm)= 110.60 110.60 110.603
01204> | RUNOFF COEFFICIENT = .99 .67 .863
01205>
01206> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01207> CN* = 83.0 Ia = Dep. Storage (Above)
01208> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01209> THAN THE STORAGE COEFFICIENT.
01210> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01211>
01212>
01213> 006:0006-----
01214> *#-----|
01215> *# Catchment North of Haldimand Road

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01216> *#-----|
01217>
01218> | DESIGN NASHYD | Area (ha)= 3.20 Curve Number (CN)=80.00
01219> | 04:204 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01220> | U.H. Tp(hrs)= .970
01221>
01222> | Unit Hyd Qpeak (cms)= .126
01223>
01224> | PEAK FLOW (cms)= .183 (i)
01225> | TIME TO PEAK (hrs)= 12.917
01226> | RUNOFF VOLUME (mm)= 68.963
01227> | TOTAL RAINFALL (mm)= 110.602
01228> | RUNOFF COEFFICIENT = .624
01229>
01230> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01231>
01232>
01233>
01234> *#-----|
01235> *# Dry End-of-Pipe SWM Facility
01236> *#-----|
01237>
01238> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01239> | IN>03:(203 ) |
01240> | OUT<05:(501 ) |
01241>
01242> |-----|
01243> | OUTFLOW STORAGE | OUTFLOW STORAGE |
01244> | (cms) (ha.m.) | (cms) (ha.m.) |
01245> |.000 .0000E+00 | .111 .9710E-01 |
01246> |.001 .1800E-02 | .125 .1188E+00 |
01247> |.002 .1040E-01 | .138 .1411E+00 |
01248> |.006 .1810E-01 | .209 .1640E+00 |
01249> |.014 .2770E-01 | .336 .1876E+00 |
01250> |.027 .3870E-01 | .505 .2117E+00 |
01251> |.044 .5090E-01 | .713 .2365E+00 |
01252> |.073 .6420E-01 | .961 .2620E+00 |
01253> |.094 .7880E-01 | .000 .0000E+00 |
01254>
01255> | ROUTING RESULTS | AREA QPEAK TPEAK R.V. |
01256> | (ha) (cms) (hrs) (mm) |
01257> | INFLOW >03: (203 ) 2.53 .636 12.000 95.432 |
01258> | OUTFLOW<05: (501 ) 2.53 .121 12.567 95.431 |
01259>
01260> | PEAK FLOW REDUCTION [Qout/Qin] (%) = 19.072 |
01261> | TIME SHIFT OF PEAK FLOW (min) = 34.00 |
01262> | MAXIMUM STORAGE USED (ha.m.) = 1130E+00 |
01263>
01264>
01265> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF |
01266> | (ha) (cms) (hrs) (mm) (cms) |
01267> | ID1 01:201 10.62 .574 13.00 70.57 .000 |
01268> | +ID2 02:202 18.25 .939 13.17 73.87 .000 |
01269> | +ID3 04:204 3.20 .183 12.92 68.96 .000 |
01270> | +ID4 05:501 2.53 .121 12.57 95.43 .000 |
01271>
01272> |-----|
01273> | SUM 06:401 34.60 1.802 13.08 73.98 .000 |
01274>
01275> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01276>
01277> 006:0009-----
01278> *#-----|
01279>
01280> 006:0002-----
01281>
01282> 006:0002-----
01283>
01284> 006:0002-----
01285>
01286> 006:0002-----
01287>
01288> 006:0002-----
01289> FINISH
01290>
01291> *****
01292> WARNINGS / ERRORS / NOTES
01293>
01294> Simulation ended on 2011-01-31 at 21:52:14
01295>

```

Grand Renewable Energy Park
Samsung Renewable Energy Inc.
Substation Facility SWM Drainage Area Characteristics and Storage Requirements

Drainage Areas
(See below)

Total Area Tributary to Basin (ha)	2.53
Tributary Area requiring quality control (ha)	2.03
MOE Quality Control Requirement Basin Design	Basic Dry Pond
¹ Quality Control Volume Requirement (m ³ /ha)	213
Extended Detention - Quality Control (m ³)	433

¹ Based on MOE guidelines and overall percent impervious

Catchment Number	Area (ha)	% Imperv (XIMP)
203	2.53	60%
Quality Control Area	2.03	75%
Quantity Control Area	2.53	60%

**Grand Renewable Energy Park
Samsung Renewable Energy Inc.
Substation Facility SWM Basin Stage-Storage-Discharge Calculations**

Rating Curve				
Elevation (m)	Discharge (m³/s)	Active Storage (m³)	Drawdown (hrs)	
			Increment	Total
199.45				
199.50	0.001	4	2.9	2.9
199.55	0.001	18	3.2	6.2
199.60	0.002	48	5.1	11.3
199.65	0.002	104	7.7	19.0
199.70	0.006	181	5.6	24.6
199.75	0.014	277	2.7	27.4
199.80	0.027	387	1.5	28.9
199.85	0.044	509	1.0	29.8
199.90	0.073	642	0.6	30.5
199.95	0.094	788	0.5	30.9
200.00	0.111	971	0.5	31.4
200.05	0.125	1,188	0.5	31.9
200.10	0.138	1,411	0.5	32.4
200.15	0.209	1,640	0.4	32.8
200.20	0.336	1,876	0.2	33.0
200.25	0.505	2,117	0.2	33.2
200.30	0.713	2,365	0.1	33.3
200.35	0.961	2,620	0.1	33.4

Volume Estimation			
Elevation (m)	Area (m²)	Int. Vol (m³)	Cum. Vol (m³)
199.45			
199.50	175	4	4
199.55	350	13	18
199.60	860	30	48
199.65	1370	56	104
199.70	1735	78	181
199.75	2100	96	277
199.80	2315	110	387
199.85	2530	121	509
199.90	2795	133	642
199.95	3060	146	788
200.00	4270	183	971
200.05	4396	217	1188
200.10	4521	223	1411
200.15	4647	229	1640
200.20	4773	236	1876
200.25	4899	242	2117
200.30	5024	248	2365
200.35	5150	254	2620

Outlet Structure Controls							
Elevation (m)	Orifice 1 (m³/s)	Orifice 2 (m³/s)	Weir (m³/s)	Total Flow (m³/s)	Parameters		
Orifice 1							
199.45	0.001			0.001	Orifice Invert Elev. (m)	Orifice Coeff.	
199.50	0.001			0.001	199.45	0.60	
199.55	0.001			0.001	Orifice Mid-point Elev. (m)	Perimeter (m)	
199.60	0.002			0.002	199.48	0.16	
199.65	0.002			0.002	Orifice Diam.(mm)	Area (m²)	
199.70	0.002	0.003		0.006	50	0.002	
199.75	0.003	0.011		0.014	Weir Coeff. (semi-circular)	Orientation	
199.80	0.003	0.024		0.027	1.62	Vertical	
199.85	0.003	0.040		0.044	Orifice 2		
199.90	0.003	0.070		0.073	Orifice Invert Elev. (m)	Orifice Coeff.	
199.95	0.004	0.090		0.094	199.65	0.60	
200.00	0.004	0.107		0.111	Orifice Mid-point Elev. (m)	Perimeter (m)	
200.05	0.004	0.121		0.125	199.83	1.10	
200.10	0.004	0.134		0.138	Orifice Diam.(mm)	Area (m²)	
200.15	0.004	0.146	0.059	0.209	350	0.096	
200.20	0.004	0.157	0.175	0.336	Weir Coeff. (semi-circular)	Orientation	
200.25	0.005	0.167	0.333	0.505	1.62	Vertical	
200.30	0.005	0.176	0.532	0.713	Emergency Overflow Weir		
200.35	0.005	0.185	0.770	0.961	Weir Invert (m)	Weir Length m	
					200.10	3.0	
					Weir Coeff. (rect.)	Side Slopes (H:V) (?:1)	
					1.70		
					Weir Coeff. (tri.)	3.0	
					0.60		

Orifice Equation Used: Orifice flow equation

$$Q = C \cdot A \cdot (2 \cdot g \cdot h)^{0.5}$$

where

- C = orifice coefficient
- A = area of orifice
- g = acceleration due to gravity
- h = head above centre line of orifice

Note: Sharp crested weir equation with equivalent linear length used for calculating orifice flow rates when head is below centre line

Sharp crested semi-circular weir equation

$$Q = C \cdot D^{2.5} \cdot (H/D)^{1.88}$$

where

- C = sharp crested semi-circular weir coefficient
- D = diameter of orifice
- H = head above orifice invert

Note: used when water elevation is below mid-point of orifice

Broad Crested Weir Equation: $Q = (C_{rectangle} \cdot L \cdot H^{3/2}) + ((C_{triangle} \cdot (8/15 \cdot (2 \cdot g)^{1/2} \cdot \tan(\Theta/2) \cdot H^{5/2}))$

where

- L = bottom width of weir
- H = head above weir invert
- S = side slopes (ratio of H:V)
- C_{triangle} = triangular weir coefficient
- C_{rectangle} = broad-crested rectangular weir coefficient

$$g = 9.81 \text{ m/s}^2$$

Θ/2 = angle formed by trapezoidal weir side slopes

Appendix C

Operations and Maintenance Facility Analyses

Grand Renewable Energy Park - O & M Facility
Samsung Renewable Energy Inc.
SCS Curve Number Determination
Existing Conditions

Site Soils: (as per Soil Survey Complex, OMAFRA / MNR, 2009)

Soil Type	Hydrologic Soil Group
Hal: Haldimand (clay)	C
Lic: Lincoln (clay)	D

TABLE OF CURVE NUMBERS (CN's)							
Land Use	Hydrologic Soil Type						
	A	AB	B	BC	C	CD	D
Meadow	50	54	58	64.5	71	74.5	78
Woodlot	50	55.3	60.5	67	73.5	76.8	80
Long Grass	55	60	65	72	79	81.5	84
Lawns	60	65.5	71	77	83	86	89
Pasture/Range	58	61.5	65	70.5	76	78.5	81
Crop	66	70	74	78	82	84	86
Fallow (Bare)	77	82	86	89	91	93	94
Wetland	50	50	50	50	50	50	50

HYDROLOGIC SOIL TYPE (%) - Existing Conditions								
Catchment	Hydrologic Soil Type							TOTAL
	A	AB	B	BC	C	CD	D	
101					100			100
102					93		7	100

LAND USE (%) - Existing Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Total
101						100			100
102		4				96			100

CURVE NUMBER (CN) - Existing Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Weighted CN
101	0	0	0	0	0	82	0	0	82
102	0	3	0	0	0	79	0	0	81

** post development catchments concerned with pervious CN values only
 ** AMC II assumed
 ** Hydrological Soil Group taken from OMAFRA website

Grand Renewable Energy Park - O & M Facility

Samsung Renewable Energy Inc.

SCS Curve Number Determination (Applies to Pervious Component of Developed Catchments Only)

Proposed Conditions

Site Soils: (as per Soil Survey Complex, OMAFRA / MNR, 2009)

Soil Type Hydrologic Soil Group

Hal: Haldimand (clay) C

Lic: Lincoln (clay) D

TABLE OF CURVE NUMBERS (CN's)							
Land Use	Hydrologic Soil Type						
	A	AB	B	BC	C	CD	D
Meadow	50	54	58	64.5	71	74.5	78
Woodlot	50	55.3	60.5	67	73.5	76.8	80
Long Grass	55	60	65	72	79	81.5	84
Lawns	60	65.5	71	77	83	86	89
Pasture/Range	58	61.5	65	70.5	76	78.5	81
Crop	66	70	74	78	82	84	86
Fallow (Bare)	77	82	86	89	91	93	94
Wetland	50	50	50	50	50	50	50
Streets, paved	98	98	98	98	98	98	98

HYDROLOGIC SOIL TYPE (%) - Proposed Conditions								
Catchment	Hydrologic Soil Type							TOTAL
	A	AB	B	BC	C	CD	D	
201a					100			100
201b					100			100
202a					100			100
202b					87		13	100
202c					97		3	100

LAND USE (%) - Proposed Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Total
201a			75			25			100
201b			100						100
202a		4	96						100
202b		7	93						100
202c			100						100

CURVE NUMBER (CN) - Proposed Conditions									
Catchment	Meadow	Woodlot	Long Grass	Lawns	Pasture Range	Crop	Fallow (Bare)	Wetland	Weighted CN
201a	0	0	59	0	0	21	0	0	80
201b	0	0	79	0	0	0	0	0	79
202a	0	3	76	0	0	0	0	0	79
202b	0	5	74	0	0	0	0	0	79
202c	0	0	79	0	0	0	0	0	79

** post development catchments concerned with pervious CN values only

** AMC II assumed

** Hydrological Soil Group taken from MTO Drainage Manual for each soil type

Grand Renewable Energy Park - O & M Facility
Samsung Renewable Energy Inc.
SWMHYMO Parameters

Existing Conditions

Catchment Number	Area Description	SWMHYMO Command	Area (ha)	CN	TIMP	XIMP	Slope (%)	Length (m)	Tc (hrs)	Tp (hrs)
101	Agricultural area draining to location of proposed access road	DESIGN NASHYD	11.1	82			0.60	500	1.29	0.78
102	Agricultural area draining to location of access road / solar module / O&M works	DESIGN NASHYD	13.8	81			0.90	650	1.29	0.77
		Total Area	24.9							

Proposed Conditions

Catchment Number	Area Description	SWMHYMO Command	Area (ha)	CN	TIMP	XIMP	Slope (%)	Length (m)	Tc (hrs)	Tp (hrs)
201a	Agricultural area to draining to diversion swale at east side of proposed access road	DESIGN NASHYD	10.9	80			0.60	470	1.25	0.75
201b	Access road right-of-way (most northerly 160 m stretch)	DESIGN STANDHYD	0.2	79	0.50	0.50	0.40	160	0.84	0.50
202a	Agricultural area draining to diversion swale at east side of access road / solar module	DESIGN NASHYD	6.3	79			0.60	470	1.25	0.75
202b	Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands	DESIGN STANDHYD	5.8	79	0.50	0.50	0.90	450	1.07	0.64
202c	Solar module area draining to diversion swale at west side of module	DESIGN NASHYD	1.7	79			0.63	150	0.70	0.42
		Total Area	24.9							

Notes:

CN calculated for pervious areas only for DESIGN STANDHYD. CN is a weighed average for DESIGN NASHYD

TIMP Total percent impervious

XIMP Directly connected percent impervious

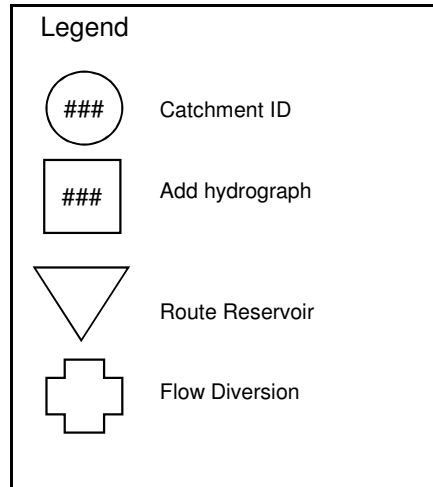
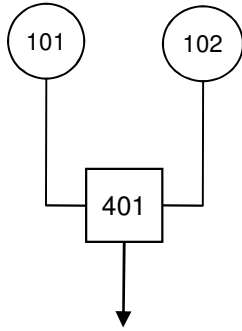
Time of Concentration calculated using the Airport Method

..... $T_c = [3.26 (1.1-C) L^{0.5}] / S^{0.33}$

Where: C = Runoff Coefficient = 0.2 for undeveloped areas
L = Length of Overland Flow (m)
= (Area/1.5)^{0.5}
S = Slope (%)

Time to Peak $T_p = 0.6T_c$

Grand Renewable Energy Park - O & M Facility
Samsung Renewable Energy Inc.
Existing Conditions SYMHYMO Schematic



```

00001> 2      Metric units
00002> #*****
00003> # Project Name : Grand Renewable Energy Park - O & M Facility
00004> # Project Number: 1610-10624
00005> # Date      : 1-27-2011
00006> # Company   : Stantec Consulting Ltd. (Kitchener)
00007> # Modeller  : S Robertson
00008> # License #  : 4730904
00009> #*****
00010> # EXISTING CONDITIONS
00011> #*****
00012> START      TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[1]
00013> *          ["si_2.STM"] <--storm filename, one per line for NSTORM time
00014> *          |-----|
00015> READ STORM  STORM_FILENAME=["STORM.001"]
00016> *          |-----|
00017> # Agricultural area draining to location of proposed access road
00018> #-----|
00019> DESIGN NASHYD ID=[1], NHYD=["101"], DT=[5]min, AREA=[11.10](ha),
00020> DWF=[0](cms), CN/C=[82], TP=[0.78]hrs,
00021> RAINFALL=[ , , , ](mm/hr), END=-1
00022> #-----|
00023> # Agricultural area draining to location of access road/solar module/O&M works
00024> #-----|
00025> DESIGN NASHYD ID=[2], NHYD=["102"], DT=[5]min, AREA=[13.8](ha),
00026> DWF=[0](cms), CN/C=[81], TP=[0.77]hrs,
00027> RAINFALL=[ , , , ](mm/hr), END=-1
00028> #-----|
00029> ADD HYD     IDsum=[3], NHYD=["401"], IDs to add=[1+2]
00030> #-----|
00031> START      TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[2]
00032> *          ["si_5.STM"] <--storm filename, one per line for NSTORM time
00033> *          |-----|
00034> START      TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[3]
00035> *          ["si_10.STM"] <--storm filename, one per line for NSTORM tim
00036> *          |-----|
00037> START      TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[4]
00038> *          ["si_25.STM"] <--storm filename, one per line for NSTORM tim
00039> *          |-----|
00040> START      TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[5]
00041> *          ["si_50.STM"] <--storm filename, one per line for NSTORM tim
00042> *          |-----|
00043> START      TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[6]
00044> *          ["si_100.STM"] <--storm filename, one per line for NSTORM ti
00045> *          |-----|
00046> FINISH
00047>
00048>
00049>
00050>
00051>
00052>
00053>
00054>
00055>
00056>
00057>
00058>
00059>
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00070>
00071>
00072>
00073>
00074>
00075>
00076>
00077>
00078>
00079>
00080>
00081>
00082>
00083> START      TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[6]
00084> *          ["si_100.STM"] <--storm filename, one per line for NSTORM ti
00085> *          |-----|
00086>
00087>
00088>
00089>
00090>
00091>
00092>
00093>
00094>
00095>
00096>
00097>
00098>
00099>
00100>

```

```

00001>-----
00002>-----
00003> SSSSS W W M M H H Y Y M M O O 999 999 -----
00004> S W W M M M H H Y Y M M O O 9 9 9 9
00005> SSSSS W W M M H H H H Y Y M M O O ## 9 9 9 9 Ver. 4.02
00006> S W W M M H H Y Y M M O O 9999 9999 July 1999
00007> SSSSS W W M M H H Y Y M M O O 9 9 9 9
00008>-----
00009> StormWater Management Hydrologic Model 999 999 -----
00010>-----
00011> *****
00012> ***** SWMHYMO-99 Ver/4.02 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHYMO-83 and OTTHYMO-89. *****
00016>-----
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Ottawa, Ontario: (613) 727-5199 *****
00019> ***** Gatineau, Quebec: (819) 243-6858 *****
00020> ***** E-Mail: swmhymo@jfsa.com *****
00021>-----
00022>-----
00023> ***** Licensed user: Stantec Consulting Ltd. (Kitchener) *****
00024> ***** Kitchener SERIAL#:4730904 *****
00025>-----
00026>-----
00027>-----
00028>-----
00029> ***** PROGRAM ARRAY DIMENSIONS *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 15000 *****
00032> ***** Max. number of flow points : 15000 *****
00033>-----
00034>-----
00035>-----
00036>-----
00037>-----
00038> ***** DETAILED OUTPUT *****
00039> ***** DATE: 2011-01-27 TIME: 22:38:46 RUN COUNTER: 001059 *****
00040> * Input filename: C:\PROGRA-1\SWMHYMO\OM_Ex.dat *
00041> * Output filename: C:\PROGRA-1\SWMHYMO\OM_Ex.out *
00042> * Summary filename: C:\PROGRA-1\SWMHYMO\OM_Ex.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047>-----
00048>-----
00049>-----
00050> 001:0001-----
00051> *#-----
00052> *# Project Name : Grand Renewable Energy Park - O & M Facility
00053> *# Project Number: 1610-10624
00054> *# Date : 1-27-2011
00055> *# Company : Stantec Consulting Ltd. (Kitchener)
00056> *# Modeller : S Robertson
00057> *# License # : 4730904
00058> *#-----
00059> *# EXISTING CONDITIONS
00060> *#-----
00061>-----
00062> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00063> |-----| Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00064> |-----| TZERO = .00 hrs on 0
00065> |-----| METOUT= 2 (output = METRIC)
00066> |-----| NRUN = 001
00067> |-----| NSTORM= 1
00068> |-----| # 1=si_2.STM
00069>-----
00070> 001:0002-----
00071>-----
00072> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_2.STM
00073> | Ptotal= 50.70 mm | Comments: SCS-II 24H 2-YEAR SIMCOE
00074>-----
00075>-----
00076>-----
00077>-----
00078>-----
00079>-----
00080>-----
00081>-----
00082>-----
00083>-----
00084>-----
00085>-----
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00094>-----
00095>-----
00096>-----
00097>-----
00098>-----
00099>-----
00100>-----
00101>-----
00102>-----
00103> 001:0003-----
00104> *#-----
00105> *# Agricultural area draining to location of proposed access road
00106> *#-----
00107>-----
00108> | DESIGN NASHYD | Area (ha)= 11.10 Curve Number (CN)=82.00
00109> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00110> |-----| U.H. Tp(hrs)= .780
00111>-----
00112> Unit Hyd Opeak (cms)= .544
00113>-----
00114> PEAK FLOW (cms)= .244 (i)
00115> TIME TO PEAK (hrs)= 12.667
00116> RUNOFF VOLUME (mm)= 23.063
00117> TOTAL RAINFALL (mm)= 50.699
00118> RUNOFF COEFFICIENT = .455
00119>-----
00120> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00121>-----
00122>-----
00123> 001:0004-----
00124> *#-----
00125> *# Agricultural area draining to location of access road/solar module/O&M works
00126> *#-----
00127>-----
00128> | DESIGN NASHYD | Area (ha)= 13.80 Curve Number (CN)=81.00
00129> | 02:102 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00130> |-----| U.H. Tp(hrs)= .770
00131>-----
00132> Unit Hyd Opeak (cms)= .685
00133>-----
00134> PEAK FLOW (cms)= .295 (i)
00135> TIME TO PEAK (hrs)= 12.667

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00136> RUNOFF VOLUME (mm)= 22.252
00137> TOTAL RAINFALL (mm)= 50.699
00138> RUNOFF COEFFICIENT = .439
00139>-----
00140> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00141>-----
00142>-----
00143>-----
00144> *#-----
00145>-----
00146> | ADD HYD (401 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00147> |-----| (ha) (cms) (hrs) (mm) (cms)
00148> |-----| ID1 01:101 11.10 .244 12.67 23.06 .000
00149> |-----| ID2 02:102 13.80 .295 12.67 22.25 .000
00150> |-----| SUM 03:401 24.90 .539 12.67 22.61 .000
00151>-----
00152>-----
00153> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00154>-----
00155>-----
00156> 001:0006-----
00157> *#-----
00158> ** END OF RUN : 1
00159>-----
00160>-----
00161>-----
00162>-----
00163>-----
00164>-----
00165>-----
00166>-----
00167> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00168> |-----| Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00169> |-----| TZERO = .00 hrs on 0
00170> |-----| METOUT= 2 (output = METRIC)
00171> |-----| NRUN = 002
00172> |-----| NSTORM= 1
00173> |-----| # 1=si_5.STM
00174>-----
00175> 002:0002-----
00176> *#-----
00177> *# Project Name : Grand Renewable Energy Park - O & M Facility
00178> *# Project Number: 1610-10624
00179> *# Date : 1-27-2011
00180> *# Company : Stantec Consulting Ltd. (Kitchener)
00181> *# Modeller : S Robertson
00182> *# License # : 4730904
00183> *#-----
00184> *# EXISTING CONDITIONS
00185> *#-----
00186>-----
00187> 002:0002-----
00188> *#-----
00189> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_5.STM
00190> | Ptotal= 66.80 mm | Comments: SCS-II 24H 5-YEAR SIMCOE
00191>-----
00192>-----
00193>-----
00194>-----
00195>-----
00196>-----
00197>-----
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00214>-----
00215>-----
00216>-----
00217>-----
00218>-----
00219>-----
00220> 002:0003-----
00221> *#-----
00222> *# Agricultural area draining to location of proposed access road
00223> *#-----
00224>-----
00225> | DESIGN NASHYD | Area (ha)= 11.10 Curve Number (CN)=82.00
00226> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00227> |-----| U.H. Tp(hrs)= .780
00228>-----
00229> Unit Hyd Opeak (cms)= .544
00230>-----
00231> PEAK FLOW (cms)= .377 (i)
00232> TIME TO PEAK (hrs)= 12.667
00233> RUNOFF VOLUME (mm)= 35.225
00234> TOTAL RAINFALL (mm)= 66.801
00235> RUNOFF COEFFICIENT = .527
00236>-----
00237> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00238>-----
00239>-----
00240> 002:0004-----
00241> *#-----
00242> *# Agricultural area draining to location of access road/solar module/O&M works
00243> *#-----
00244>-----
00245> | DESIGN NASHYD | Area (ha)= 13.80 Curve Number (CN)=81.00
00246> | 02:102 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00247> |-----| U.H. Tp(hrs)= .770
00248>-----
00249> Unit Hyd Opeak (cms)= .685
00250>-----
00251> PEAK FLOW (cms)= .458 (i)
00252> TIME TO PEAK (hrs)= 12.667
00253> RUNOFF VOLUME (mm)= 34.146
00254> TOTAL RAINFALL (mm)= 66.801
00255> RUNOFF COEFFICIENT = .511
00256>-----
00257> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00258>-----
00259>-----
00260> 002:0005-----
00261> *#-----
00262>-----
00263> | ADD HYD (401 ) | ID: NHYD AREA OPEAK TPEAK R.V. DWF
00264> |-----| (ha) (cms) (hrs) (mm) (cms)
00265> |-----| ID1 01:101 11.10 .377 12.67 35.23 .000
00266> |-----| ID2 02:102 13.80 .458 12.67 34.15 .000
00267> |-----| SUM 03:401 24.90 .835 12.67 34.63 .000
00268>-----
00269>-----
00270> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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00541> *# Project Number: 1610-10624
00542> *# Date : 1-27-2011
00543> *# Company : Stantec Consulting Ltd. (Kitchener)
00544> *# Modeller : S Robertson
00545> *# License # : 4730904
00546> *****
00547> *# EXISTING CONDITIONS
00548> *****
00549> -----
00550> 005:0002-----
00551> -----
00552> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_50.STM
00553> | Ptotal= 100.70 mm | Comments: SCS-II 24H 50-YEAR SIMCOE
00554> -----
00555> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00556> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00557> .25 1.108 | 6.25 2.014 | 12.25 4.501 | 18.25 1.813
00558> .50 1.108 | 6.50 2.014 | 12.50 4.501 | 18.50 1.813
00559> .75 1.108 | 6.75 2.014 | 12.75 4.501 | 18.75 1.813
00560> 1.00 1.108 | 7.00 2.014 | 13.00 4.501 | 19.00 1.813
00561> 1.25 1.108 | 7.25 2.014 | 13.25 4.501 | 19.25 1.813
00562> 1.50 1.108 | 7.50 2.014 | 13.50 4.501 | 19.50 1.813
00563> 1.75 1.108 | 7.75 2.014 | 13.75 4.501 | 19.75 1.813
00564> 2.00 1.108 | 8.00 2.014 | 14.00 4.501 | 20.00 1.813
00565> 2.25 1.309 | 8.25 2.719 | 14.25 3.021 | 20.25 1.208
00566> 2.50 1.309 | 8.50 2.719 | 14.50 3.021 | 20.50 1.208
00567> 2.75 1.309 | 8.75 2.719 | 14.75 3.021 | 20.75 1.208
00568> 3.00 1.309 | 9.00 2.719 | 15.00 3.021 | 21.00 1.208
00569> 3.25 1.309 | 9.25 2.222 | 15.25 3.021 | 21.25 1.208
00570> 3.50 1.309 | 9.50 3.222 | 15.50 3.021 | 21.50 1.208
00571> 3.75 1.309 | 9.75 3.625 | 15.75 3.021 | 21.75 1.208
00572> 4.00 1.309 | 10.00 3.625 | 16.00 3.021 | 22.00 1.208
00573> 4.25 1.611 | 10.25 4.632 | 16.25 1.813 | 22.25 1.208
00574> 4.50 1.611 | 10.50 4.632 | 16.50 1.813 | 22.50 1.208
00575> 4.75 1.611 | 10.75 6.243 | 16.75 1.813 | 22.75 1.208
00576> 5.00 1.611 | 11.00 6.243 | 17.00 1.813 | 23.00 1.208
00577> 5.25 1.611 | 11.25 9.667 | 17.25 1.813 | 23.25 1.208
00578> 5.50 1.611 | 11.50 9.667 | 17.50 1.813 | 23.50 1.208
00579> 5.75 1.611 | 11.75 11.173 | 17.75 1.813 | 23.75 1.208
00580> 6.00 1.611 | 12.00 11.173 | 18.00 1.813 | 24.00 1.208
00581> -----
00582> -----
00583> 005:0004-----
00584> *#-----
00585> *# Agricultural area draining to location of proposed access road
00586> *#-----
00587> -----
00588> | DESIGN NASHYD | Area (ha)= 11.10 Curve Number (CN)=82.00
00589> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00590> | U.H. Tp(hrs)= .780
00591> -----
00592> Unit Hyd Opeak (cms)= .544
00593> -----
00594> PEAK FLOW (cms)= .687 (i)
00595> TIME TO PEAK (hrs)= 12.667
00596> RUNOFF VOLUME (mm)= 63.505
00597> TOTAL RAINFALL (mm)= 100.699
00598> RUNOFF COEFFICIENT = .631
00599> -----
00600> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00601> -----
00602> -----
00603> 005:0004-----
00604> *#-----
00605> *# Agricultural area draining to location of access road/solar module/O&M works
00606> *#-----
00607> -----
00608> | DESIGN NASHYD | Area (ha)= 13.80 Curve Number (CN)=81.00
00609> | 02:102 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00610> | U.H. Tp(hrs)= .770
00611> -----
00612> Unit Hyd Opeak (cms)= .685
00613> -----
00614> PEAK FLOW (cms)= .841 (i)
00615> TIME TO PEAK (hrs)= 12.667
00616> RUNOFF VOLUME (mm)= 61.976
00617> TOTAL RAINFALL (mm)= 100.699
00618> RUNOFF COEFFICIENT = .615
00619> -----
00620> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00621> -----
00622> -----
00623> 005:0005-----
00624> *#-----
00625> -----
00626> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00627> |-----|-----|-----|-----|-----|-----|
00628> | 01:101 | (ha) | (cms) | (hrs) | (mm) | (cms) |
00629> | +ID2 02:102 | 13.80 | .841 | 12.67 | 61.98 | .000 |
00630> |-----|-----|-----|-----|-----|-----|
00631> | SUM 03:401 | 24.90 | 1.528 | 12.67 | 62.66 | .000 |
00632> -----
00633> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00634> -----
00635> -----
00636> 005:0006-----
00637> *#-----
00638> -----
00639> 005:0002-----
00640> -----
00641> 005:0002-----
00642> -----
00643> 005:0002-----
00644> -----
00645> 005:0002-----
00646> ** END OF RUN : 5
00647> -----
00648> *****
00649> -----
00650> -----
00651> -----
00652> -----
00653> -----
00654> -----
00655> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00656> |-----| Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00657> TZERO= .00 hrs on 0
00658> METOUT= 2 (output = METRIC)
00659> NRUN = 006
00660> NSTORM= 1
00661> # 1=si_100.STM
00662> -----
00663> 006:0002-----
00664> *****
00665> *# Project Name : Grand Renewable Energy Park - O & M Facility
00666> *# Project Number: 1610-10624
00667> *# Date : 1-27-2011
00668> *# Company : Stantec Consulting Ltd. (Kitchener)
00669> *# Modeller : S Robertson
00670> *# License # : 4730904
00671> *****
00672> *# EXISTING CONDITIONS
00673> *****
00674> -----
00675> 006:0002-----

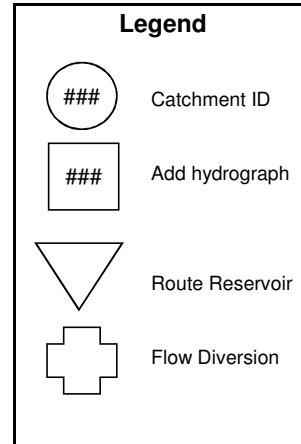
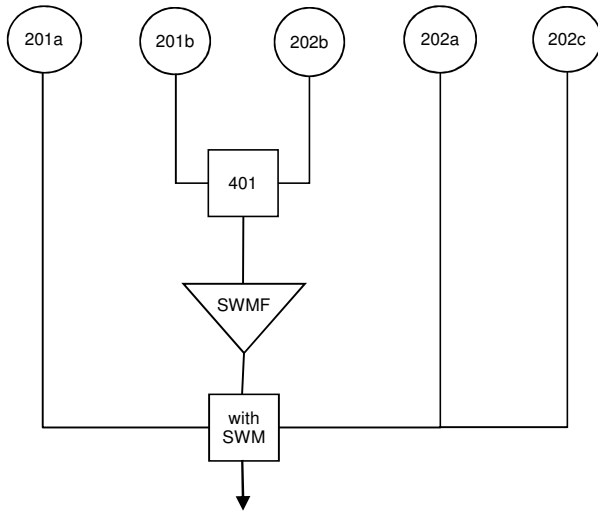
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00676> -----
00677> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_100.STM
00678> | Ptotal= 110.60 mm | Comments: SCS-II 24H 100-YEAR SIMCOE
00679> -----
00680> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00681> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00682> .25 1.217 | 6.25 2.212 | 12.25 15.926 | 18.25 1.991
00683> .50 1.217 | 6.50 2.212 | 12.50 15.926 | 18.50 1.991
00684> .75 1.217 | 6.75 2.212 | 12.75 8.184 | 18.75 1.991
00685> 1.00 1.217 | 7.00 2.212 | 13.00 8.184 | 19.00 1.991
00686> 1.25 1.217 | 7.25 2.212 | 13.25 5.751 | 19.25 1.991
00687> 1.50 1.217 | 7.50 2.212 | 13.50 6.194 | 19.50 1.991
00688> 1.75 1.217 | 7.75 2.212 | 13.75 4.645 | 19.75 1.991
00689> 2.00 1.217 | 8.00 2.212 | 14.00 4.645 | 20.00 1.991
00690> 2.25 1.438 | 8.25 2.986 | 14.25 3.318 | 20.25 1.327
00691> 2.50 1.438 | 8.50 2.986 | 14.50 3.318 | 20.50 1.327
00692> 2.75 1.438 | 8.75 2.986 | 14.75 3.318 | 20.75 1.327
00693> 3.00 1.438 | 9.00 2.986 | 15.00 3.318 | 21.00 1.327
00694> 3.25 1.438 | 9.25 3.539 | 15.25 3.318 | 21.25 1.327
00695> 3.50 1.438 | 9.50 3.539 | 15.50 3.318 | 21.50 1.327
00696> 3.75 1.438 | 9.75 3.982 | 15.75 3.318 | 21.75 1.327
00697> 4.00 1.438 | 10.00 3.982 | 16.00 3.318 | 22.00 1.327
00698> 4.25 1.770 | 10.25 5.088 | 16.25 1.991 | 22.25 1.327
00699> 4.50 1.770 | 10.50 5.088 | 16.50 1.991 | 22.50 1.327
00700> 4.75 1.770 | 10.75 6.857 | 16.75 1.991 | 22.75 1.327
00701> 5.00 1.770 | 11.00 6.857 | 17.00 1.991 | 23.00 1.327
00702> 5.25 1.770 | 11.25 10.618 | 17.25 1.991 | 23.25 1.327
00703> 5.50 1.770 | 11.50 10.618 | 17.50 1.991 | 23.50 1.327
00704> 5.75 1.770 | 11.75 46.010 | 17.75 1.991 | 23.75 1.327
00705> 6.00 1.770 | 12.00 122.102 | 18.00 1.991 | 24.00 1.327
00706> -----
00707> -----
00708> 006:0003-----
00709> *#-----
00710> *# Agricultural area draining to location of proposed access road
00711> *#-----
00712> -----
00713> | DESIGN NASHYD | Area (ha)= 11.10 Curve Number (CN)=82.00
00714> | 01:101 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00715> | U.H. Tp(hrs)= .780
00716> -----
00717> Unit Hyd Opeak (cms)= .544
00718> -----
00719> PEAK FLOW (cms)= .783 (i)
00720> TIME TO PEAK (hrs)= 12.667
00721> RUNOFF VOLUME (mm)= 72.203
00722> TOTAL RAINFALL (mm)= 110.602
00723> RUNOFF COEFFICIENT = .653
00724> -----
00725> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00726> -----
00727> -----
00728> -----
00729> *#-----
00730> *# Agricultural area draining to location of access road/solar module/O&M works
00731> *#-----
00732> -----
00733> | DESIGN NASHYD | Area (ha)= 13.80 Curve Number (CN)=81.00
00734> | 02:102 DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00735> | U.H. Tp(hrs)= .770
00736> -----
00737> Unit Hyd Opeak (cms)= .685
00738> -----
00739> PEAK FLOW (cms)= .959 (i)
00740> TIME TO PEAK (hrs)= 12.667
00741> RUNOFF VOLUME (mm)= 70.566
00742> TOTAL RAINFALL (mm)= 110.602
00743> RUNOFF COEFFICIENT = .638
00744> -----
00745> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00746> -----
00747> -----
00748> 006:0005-----
00749> *#-----
00750> -----
00751> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00752> |-----|-----|-----|-----|-----|-----|
00753> | ID1 01:101 | (ha) | (cms) | (hrs) | (mm) | (cms) |
00754> | +ID2 02:102 | 13.80 | .959 | 12.67 | 70.57 | .000 |
00755> |-----|-----|-----|-----|-----|-----|
00756> | SUM 03:401 | 24.90 | 1.742 | 12.67 | 71.30 | .000 |
00757> -----
00758> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00759> -----
00760> -----
00761> 006:0006-----
00762> *#-----
00763> -----
00764> 006:0002-----
00765> -----
00766> 006:0002-----
00767> -----
00768> 006:0002-----
00769> -----
00770> 006:0002-----
00771> -----
00772> 006:0002-----
00773> FINISH
00774> -----
00775> *****
00776> WARNINGS / ERRORS / NOTES
00777> -----
00778> Simulation ended on 2011-01-27 at 22:38:46
00779> -----
00780> -----

```

Grand Renewable Energy Park - O & M Facility
Samsung Renewable Energy Inc.
Proposed Conditions SYMHYMO Schematic



```

00001> 2 Metric units
00002> #*****
00003> # Project Name : Grand Renewable Energy Park - O & M Facility
00004> # Project Number: 1610-10624
00005> # Date : 1-27-2011
00006> # Company : Stantec Consulting Ltd. (Kitchener)
00007> # Modeller : S Robertson
00008> # License # : 4730904
00009> #*****
00010> # PROPOSED CONDITIONS
00011> #*****
00012> START TZERO=[0.0], METOUT=[2], NSTORM=[1], NRUN=[1]
00013> * ["si_2.STM"] <--storm filename, one per line for NSTORM time
00014> *
00015> READ STORM STORM_FILENAME=["STORM.001"]
00016> #-----
00017> # Agricultural area to draining to diversion swale at east side of access road
00018> #-----
00019> DESIGN NASHYD ID=[1], NHYD=["201a"], DT=[5]min, AREA=[10.9] (ha),
00020> DWF=[0] (cms), CN/C=[80], TP=[0.75]hrs,
00021> RAINFALL=[ , , , ] (mm/hr), END=-1
00022> #-----
00023> # Access road right-of-way (most northerly 160 m stretch)
00024> #-----
00025> DESIGN STANDHYD ID=[2], NHYD=["201b"], DT=[1]min, AREA=[0.2] (ha),
00026> XIMP=[0.5], TIMP=[0.5], DWF=[0] (cms), LOSS=[2], CN=[79],
00027> SLOPE=[0.6] (%), RAINFALL=[ , , , ] (mm/hr), END=-1
00028> #-----
00029> # Agricultural area draining to diversion swale at access road/solar module
00030> #-----
00031> DESIGN NASHYD ID=[3], NHYD=["202a"], DT=[5]min, AREA=[6.3] (ha),
00032> DWF=[0] (cms), CN/C=[79], TP=[0.75]hrs,
00033> RAINFALL=[ , , , ] (mm/hr), END=-1
00034> #-----
00035> # Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands
00036> #-----
00037> DESIGN STANDHYD ID=[4], NHYD=["202b"], DT=[1]min, AREA=[5.8] (ha),
00038> XIMP=[0.5], TIMP=[0.5], DWF=[0] (cms), LOSS=[2], CN=[79],
00039> SLOPE=[0.9] (%), RAINFALL=[ , , , ] (mm/hr), END=-1
00040> #-----
00041> # Sum of flows to constructed wetland SWMF
00042> #-----
00043> ADD HYD IDsum=[5], NHYD=["401"], IDs to add=[2+4]
00044> #-----
00045> # Constructed wetland SWMF
00046> #-----
00047> ROUTE RESERVOIR IDout=[6], NHYD=["DP1"], IDin=[5],
00048> RDT=[1] (min)
00049> TABLE of ( OUTFLOW-STORAGE ) values
00050> (cms) - (ha-m)
00051> [ 0.0 , 0.0 ]
00052> [ 0.003 , 0.0289 ]
00053> [ 0.005 , 0.0595 ]
00054> [ 0.016 , 0.0916 ]
00055> [ 0.049 , 0.1253 ]
00056> [ 0.081 , 0.1607 ]
00057> [ 0.103 , 0.1976 ]
00058> [ 0.121 , 0.2361 ]
00059> [ 0.136 , 0.2763 ]
00060> [ 0.150 , 0.3180 ]
00061> [ 0.459 , 0.3613 ]
00062> [ 1.087 , 0.4063 ]
00063> [ 2.002 , 0.4528 ]
00064> [ -1 , -1 ] (max twenty pts)
00065> IDovf=[ , ], NHYDovf=[ ]
00066> #-----
00067> # Solar module area draining to diversion swale at west side of module
00068> #-----
00069> DESIGN NASHYD ID=[7], NHYD=["202c"], DT=[5]min, AREA=[1.70] (ha),
00070> DWF=[0] (cms), CN/C=[79], TP=[0.42]hrs,
00071> RAINFALL=[ , , , ] (mm/hr), END=-1
00072> #-----
00073> ADD HYD IDsum=[8], NHYD=["woutSWM"], IDs to add=[1+2+3+4+7]
00074> #-----
00075> ADD HYD IDsum=[10], NHYD=["withSWM"], IDs to add=[1+3+6+7]
00076> #-----
00077> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[2]
00078> * ["si_5.STM"] <--storm filename, one per line for NSTORM time
00079> *
00080> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[3]
00081> * ["si_10.STM"] <--storm filename, one per line for NSTORM tim
00082> *
00083> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[4]
00084> * ["si_25.STM"] <--storm filename, one per line for NSTORM tim
00085> *
00086> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[5]
00087> * ["si_50.STM"] <--storm filename, one per line for NSTORM tim
00088> *
00089> START TZERO=[0.0]hrs or date, METOUT=[2], NSTORM=[1], NRUN=[6]
00090> * ["si_100.STM"] <--storm filename, one per line for NSTORM ti
00091> *
00092> FINISH
00093>
00094>
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00001>-----
00002>
00003> SSSSS W W M M H H Y Y M M O O 999 999 -----
00004> S W W M M M H H Y Y M M O O 9 9 9 9
00005> SSSSS W W M M M H H H Y Y M M O O ## 9 9 9 9 Ver. 4.02
00006> S W W M M H H H Y Y M M O O 9999 9999 July 1999
00007> SSSSS W W M M H H Y Y M M O O 9 9 9 9 -----
00008> ***** Ottawa, Ontario: (613) 727-5199 *****
00009> StormWater Management Hydrologic Model 999 999 -----
00010>
00011> *****
00012> ***** SWMHYMO-99 Ver/4.02 *****
00013> ***** A single event and continuous hydrologic simulation model *****
00014> ***** based on the principles of HYMO and its successors *****
00015> ***** OTTHYMO-83 and OTTHYMO-89. *****
00016> *****
00017> ***** Distributed by: J.F. Sabourin and Associates Inc. *****
00018> ***** Gatineau, Quebec: (819) 243-6858 *****
00019> ***** E-Mail: swmhyo@jfsa.com *****
00020> *****
00021> *****
00022>
00023> *****
00024> ***** Licensed user: Stantec Consulting Ltd. (Kitchener) *****
00025> ***** Kitchener SERIAL#:4730904 *****
00026> *****
00027> *****
00028> ***** PROGRAM ARRAY DIMENSIONS *****
00029> *****
00030> ***** Maximum value for ID numbers : 10 *****
00031> ***** Max. number of rainfall points: 15000 *****
00032> ***** Max. number of flow points : 15000 *****
00033> *****
00034> *****
00035> *****
00036> ***** DETAILED OUTPUT *****
00037> *****
00038> ***** DATE: 2011-01-28 TIME: 15:21:06 RUN COUNTER: 001065 *****
00039> *****
00040> * Input filename: C:\PROGRA-1\SWMHYMO\OM_Prop.dat *
00041> * Output filename: C:\PROGRA-1\SWMHYMO\OM_Prop.out *
00042> * Summary filename: C:\PROGRA-1\SWMHYMO\OM_Prop.sum *
00043> * User comments: *
00044> * 1: *
00045> * 2: *
00046> * 3: *
00047> *****
00048> *****
00049> *****
00050> 001:0001-----
00051> # *****
00052> # Project Name : Grand Renewable Energy Park - O & M Facility
00053> # Project Number: 161-0624
00054> # Date : 1-27-2011
00055> # Company : Stantec Consulting Ltd. (Kitchener)
00056> # Modeller : S Robertson
00057> # License # : 4730904
00058> # *****
00059> # PROPOSED CONDITIONS
00060> # *****
00061> # *****
00062> | START | Project dir : C:\PROGRA-1\SWMHYMO\
00063> | Rainfall dir : C:\PROGRA-1\SWMHYMO\
00064> | TZERO = .00 hrs on
00065> | METOUT= 2 (output = METRIC)
00066> | NRUN = 001
00067> | NSTORM= 1
00068> | # 1=si_2.STM
00069> # *****
00070> 001:0002-----
00071> # *****
00072> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_2.STM
00073> | Ptotal= 50.70 mm | Comments: SCS-II 24H 2-YEAR SIMCOE
00074> # *****
00075> # TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00076> # hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00077> .25 .558 | 6.25 1.014 | 12.25 7.301 | 18.25 .913
00078> .50 .558 | 6.50 1.014 | 12.50 7.301 | 18.50 .913
00079> .75 .558 | 6.75 1.014 | 12.75 7.352 | 18.75 .913
00080> 1.00 .558 | 7.00 1.014 | 13.00 7.352 | 19.00 .913
00081> 1.25 .558 | 7.25 1.014 | 13.25 7.636 | 19.25 .913
00082> 1.50 .558 | 7.50 1.014 | 13.50 7.839 | 19.50 .913
00083> 1.75 .558 | 7.75 1.014 | 13.75 7.829 | 19.75 .913
00084> 2.00 .558 | 8.00 1.014 | 14.00 7.219 | 20.00 .913
00085> 2.25 .659 | 8.25 1.369 | 14.25 1.521 | 20.25 .608
00086> 2.50 .659 | 8.50 1.369 | 14.50 1.521 | 20.50 .608
00087> 2.75 .659 | 8.75 1.369 | 14.75 1.521 | 20.75 .608
00088> 3.00 .659 | 9.00 1.369 | 15.00 1.521 | 21.00 .608
00089> 3.25 .659 | 9.25 1.622 | 15.25 1.521 | 21.25 .608
00090> 3.50 .659 | 9.50 1.622 | 15.50 1.521 | 21.50 .608
00091> 3.75 .659 | 9.75 1.825 | 15.75 1.521 | 21.75 .608
00092> 4.00 .659 | 10.00 1.825 | 16.00 1.521 | 22.00 .608
00093> 4.25 .811 | 10.25 2.332 | 16.25 .913 | 22.25 .608
00094> 4.50 .811 | 10.50 3.332 | 16.50 .913 | 22.50 .608
00095> 4.75 .811 | 10.75 3.143 | 16.75 .913 | 22.75 .608
00096> 5.00 .811 | 11.00 3.143 | 17.00 .913 | 23.00 .608
00097> 5.25 .811 | 11.25 4.867 | 17.25 .913 | 23.25 .608
00098> 5.50 .811 | 11.50 4.867 | 17.50 .913 | 23.50 .608
00099> 5.75 .811 | 11.75 21.091 | 17.75 .913 | 23.75 .608
00100> 6.00 .811 | 12.00 55.973 | 18.00 .913 | 24.00 .608
00101> # *****
00102> # *****
00103> 001:0003-----
00104> # *****
00105> # Agricultural area to draining to diversion swale at east side of access road
00106> # *****
00107> # *****
00108> | DESIGN NASHYD | Area (ha)= 10.90 Curve Number (CN)=80.00
00109> | 01:201a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00110> | U.H. Tp(hrs)= .750
00111> # *****
00112> | Unit Hyd Qpeak (cms)= .555
00113> # *****
00114> | PEAK FLOW (cms)= .229 (i)
00115> | TIME TO PEAK (hrs)= 12.667
00116> | RUNOFF VOLUME (mm)= 21.478
00117> | TOTAL RAINFALL (mm)= 50.699
00118> | RUNOFF COEFFICIENT = .424
00119> # *****
00120> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00121> # *****
00122> # *****
00123> 001:0004-----
00124> # *****
00125> # Access road right-of-way (most northerly 160 m stretch)
00126> # *****
00127> # *****
00128> | DESIGN STANDHYD | Area (ha)= .20
00129> | 02:201b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
00130> # *****
00131> # ***** IMPERVIOUS PERVIOUS (i)
00132> | Surface Area (ha)= .10 .10
00133> | Dep. Storage (mm)= .80 1.50
00134> | Average Slope (%)= .60 .60
00135> | Length (m)= 36.51 40.00

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00136> Mannings n = .013 .250
00137>
00138> Max.eff.Inten.(mm/hr)= 55.97 20.81
00139> over (min) = 2.00 21.00
00140> Storage Coeff. (min)= 2.05 (ii) 21.03 (ii)
00141> Unit Hyd. Tpeak (min)= 2.00 21.00
00142> Unit Hyd. peak (cms)= .55 .05 *TOTALS*
00143>
00144> PEAK FLOW (cms)= .02 .00 .018 (iii)
00145> TIME TO PEAK (hrs)= 12.00 12.23 12.000
00146> RUNOFF VOLUME (mm)= 49.90 20.73 35.319
00147> TOTAL RAINFALL (mm)= 50.70 50.70 50.699
00148> RUNOFF COEFFICIENT = .98 .41 .697
00149>
00150> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00151> CN* = 79.0 Ia = Dep. Storage (Above)
00152> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00153> THAN THE STORAGE COEFFICIENT.
00154> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00155>
00156>
00157> 001:0005-----
00158> # *****
00159> # Agricultural area draining to diversion swale at access road/solar module
00160> # *****
00161> # *****
00162> | DESIGN NASHYD | Area (ha)= 6.30 Curve Number (CN)=79.00
00163> | 03:202a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00164> | U.H. Tp(hrs)= .750
00165> # *****
00166> | Unit Hyd Qpeak (cms)= .321
00167> # *****
00168> | PEAK FLOW (cms)= .127 (i)
00169> | TIME TO PEAK (hrs)= 12.667
00170> | RUNOFF VOLUME (mm)= 20.738
00171> | TOTAL RAINFALL (mm)= 50.699
00172> | RUNOFF COEFFICIENT = .409
00173> # *****
00174> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00175> # *****
00176> # *****
00177> 001:0006-----
00178> # *****
00179> # Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands
00180> # *****
00181> # *****
00182> | DESIGN STANDHYD | Area (ha)= 5.80
00183> | 04:202b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
00184> # *****
00185> # ***** IMPERVIOUS PERVIOUS (i)
00186> | Surface Area (ha)= 2.90 2.90
00187> | Dep. Storage (mm)= .80 1.50
00188> | Average Slope (%)= .90 .90
00189> | Length (m)= 196.64 40.00
00190> | Mannings n = .013 .250
00191> # *****
00192> | Max.eff.Inten.(mm/hr)= 55.97 20.19
00193> | over (min) = 5.00 22.00
00194> | Storage Coeff (min)= 4.99 (ii) 22.00 (ii)
00195> | Unit Hyd. Tpeak (min)= 5.00 22.00
00196> | Unit Hyd. peak (cms)= .23 .05 *TOTALS*
00197> # *****
00198> | PEAK FLOW (cms)= .43 .10 .487 (iii)
00199> | TIME TO PEAK (hrs)= 12.00 12.25 12.000
00200> | RUNOFF VOLUME (mm)= 49.90 20.73 35.319
00201> | TOTAL RAINFALL (mm)= 50.70 50.70 50.699
00202> | RUNOFF COEFFICIENT = .98 .41 .697
00203> # *****
00204> | (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00205> | CN* = 79.0 Ia = Dep. Storage (Above)
00206> | (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00207> | THAN THE STORAGE COEFFICIENT.
00208> | (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00209> # *****
00210> # *****
00211> 001:0007-----
00212> # *****
00213> # Sum of flows to constructed wetland SWMF
00214> # *****
00215> # *****
00216> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00217> | (ha) (cms) (hrs) (mm) (cms)
00218> | ID2 02:201b .20 .018 12.00 35.32 .000
00219> | +ID2 04:202b 5.80 .487 12.00 35.32 .000
00220> # *****
00221> | SUM 05:401 6.00 .504 12.00 35.32 .000
00222> # *****
00223> # *****
00224> # *****
00225> # *****
00226> 001:0008-----
00227> # *****
00228> # Constructed wetland SWMF
00229> # *****
00230> # *****
00231> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00232> | IN>05: (401 ) |
00233> | OUT<06: (DP1 ) |
00234> # *****
00235> | OUTFLOW STORAGE OUTFLOW STORAGE
00236> | (cms) (ha.m.) | (cms) (ha.m.)
00237> | .000 .0000E+00 | .121 .2361E+00
00238> | .003 .2890E-01 | .136 .2763E+00
00239> | .005 .5950E-01 | .150 .3180E+00
00240> | .016 .9160E-01 | .459 .3613E+00
00241> | .049 .1253E+00 | 1.087 .4063E+00
00242> | .081 .1607E+00 | 2.002 .4528E+00
00243> | .103 .1976E+00 | .000 .0000E+00
00244> # *****
00245> # ***** ROUTING RESULTS AREA QPEAK TPEAK R.V.
00246> | (ha) (cms) (hrs) (mm)
00247> | INFLOW >05: (401 ) 6.00 .504 12.000 35.319
00248> | OUTFLOW<06: (DP1 ) 6.00 .052 13.183 35.316
00249> # *****
00250> # ***** PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.300
00251> # ***** TIME SHIFT OF PEAK FLOW (min) = 71.00
00252> # ***** MAXIMUM STORAGE USED (ha.m.) = .1286E+00
00253> # *****
00254> 001:0009-----
00255> # Solar module area draining to diversion swale at west side of module
00256> # *****
00257> # *****
00258> | DESIGN NASHYD | Area (ha)= 1.70 Curve Number (CN)=79.00
00259> | 07:202c DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00260> | U.H. Tp(hrs)= .420
00261> # *****
00262> # *****
00263> | Unit Hyd Qpeak (cms)= .155
00264> # *****
00265> | PEAK FLOW (cms)= .052 (i)
00266> | TIME TO PEAK (hrs)= 12.250
00267> | RUNOFF VOLUME (mm)= 20.738
00268> | TOTAL RAINFALL (mm)= 50.699
00269> | RUNOFF COEFFICIENT = .409
00270> # *****
00271> | (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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00271>
00272>
00273> 001:0010-----|-----|-----|-----|-----|-----|-----|-----|
00274> *#-----|-----|-----|-----|-----|-----|-----|-----|
00275>
00276> | ADD HYD (woutSW) | ID: NHYD | AREA | QPEAK | TPEAK | R.V. | DWF
00277> |-----|-----|-----|-----|-----|-----|-----|
00278> | ID1 01:201a | (ha) | (cms) | (hrs) | (mm) | (cms) |
00279> | +ID2 02:201b | 10.90 | .229 | 12.67 | 21.48 | .000 |
00280> | +ID3 03:202a | .20 | .018 | 12.00 | 35.32 | .000 |
00281> | +ID4 04:202b | 6.30 | .127 | 12.67 | 20.74 | .000 |
00282> | +ID5 07:202c | 5.80 | .487 | 12.00 | 35.32 | .000 |
00283> | +ID5 07:202c | 1.70 | .052 | 12.25 | 20.74 | .000 |
00284> |-----|-----|-----|-----|-----|-----|-----|
00285> | SUM 08:woutSW | 24.90 | .680 | 12.02 | 24.58 | .000 |
00286>
00287> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00288>
00289> 001:0011-----|-----|-----|-----|-----|-----|-----|-----|
00290> *#-----|-----|-----|-----|-----|-----|-----|-----|
00291>
00292> | ADD HYD (withSW) | ID: NHYD | AREA | QPEAK | TPEAK | R.V. | DWF
00293> |-----|-----|-----|-----|-----|-----|-----|
00294> | ID1 01:201a | (ha) | (cms) | (hrs) | (mm) | (cms) |
00295> | +ID2 03:202a | 10.90 | .229 | 12.67 | 21.48 | .000 |
00296> | +ID3 06:DP1 | 6.30 | .127 | 12.67 | 20.74 | .000 |
00297> | +ID4 06:DP1 | 6.00 | .052 | 13.18 | 35.32 | .000 |
00298> | +ID4 07:202c | 1.70 | .052 | 12.25 | 20.74 | .000 |
00299> |-----|-----|-----|-----|-----|-----|-----|
00300> | SUM 10:withSW | 24.90 | .442 | 12.67 | 24.57 | .000 |
00301>
00302> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00303>
00304> 001:0012-----|-----|-----|-----|-----|-----|-----|-----|
00305> *#-----|-----|-----|-----|-----|-----|-----|-----|
00306> ** END OF RUN : 1
00307>
00308>
00309>
00310>
00311>
00312>
00313>
00314>
00315> | START | Project dir.: C:\PROGRA-1\SWMHYM\
00316> | Rainfall dir.: C:\PROGRA-1\SWMHYM\
00317> | TZERO = .00 hrs on 0
00318> | METOUT= 2 (output = METRIC)
00319> | NRUN = 002
00320> | NSTORM= 1
00321> | # 1=si 5.STM
00322>
00323> 002:0002-----|-----|-----|-----|-----|-----|-----|-----|
00324> *#-----|-----|-----|-----|-----|-----|-----|-----|
00325> *# Project Name : Grand Renewable Energy Park - O & M Facility
00326> *# Project Number: 1610-10624
00327> *# Date : 1-27-2011
00328> *# Company : Stantec Consulting Ltd. (Kitchener)
00329> *# Modeller : S Robertson
00330> *# License # : 4730904
00331> *#-----|-----|-----|-----|-----|-----|-----|-----|
00332> *# PROPOSED CONDITIONS
00333> *#-----|-----|-----|-----|-----|-----|-----|-----|
00334>
00335> 002:0002-----|-----|-----|-----|-----|-----|-----|-----|
00336>
00337> | READ STORM | Filename: C:\PROGRA-1\SWMHYM\si_5.STM
00338> | Ptotal= 66.80 mm | Comments: SCS-II 24H 5-YEAR SIMCOE
00339>
00340>
00341>
00342>
00343>
00344>
00345>
00346>
00347>
00348>
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00358>
00359>
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00363>
00364>
00365>
00366>
00367>
00368> 002:0003-----|-----|-----|-----|-----|-----|-----|-----|
00369> *#-----|-----|-----|-----|-----|-----|-----|-----|
00370> *# Agricultural area to draining to diversion swale at east side of access road
00371> *#-----|-----|-----|-----|-----|-----|-----|-----|
00372>
00373> | DESIGN NASHYD | Area (ha)= 10.90 | Curve Number (CN)=80.00
00374> | ID1:201a DT= 5.00 | Ia (mm)= 1.500 | # of Linear Res.(N)= 3.00
00375> |-----|-----|-----|-----|-----|-----|-----|-----|
00376> | U.H. Tp(hrs)= .750 |
00377>
00378> Unit Hyd Qpeak (cms)= .555
00379> PEAK FLOW (cms)= .356 (i)
00380> TIME TO PEAK (hrs)= 12.667
00381> RUNOFF VOLUME (mm)= 33.107
00382> TOTAL RAINFALL (mm)= 66.801
00383> RUNOFF COEFFICIENT = .496
00384>
00385> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00386>
00387>
00388> 002:0004-----|-----|-----|-----|-----|-----|-----|-----|
00389> *#-----|-----|-----|-----|-----|-----|-----|-----|
00390> *# Access road right-of-way (most northerly 160 m stretch)
00391> *#-----|-----|-----|-----|-----|-----|-----|-----|
00392>
00393> | DESIGN STANDHYD | Area (ha)= .20
00394> | ID2:201b DT= 1.00 | Total Imp(%)= 50.00 | Dir. Conn.(%)= 50.00
00395> |-----|-----|-----|-----|-----|-----|-----|-----|
00396>
00397> | IMPERVIOUS | PERVIOUS (i)
00398> | Surface Area (ha)= .10 | .10
00399> | Dep. Storage (mm)= .80 | 1.50
00400> | Average Slope (%)= .60 | .60
00401> | Length (m)= 36.51 | 40.00
00402> | Mannings n = .013 | .250
00403>
00404> Max.eff.Inten.(mm/hr)= 73.75 | 37.28
00405> over (min) = 2.00 | 17.00
00406> Storage Coeff. (min)= 1.84 (ii) | 16.87 (ii)

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00406> Unit Hyd. Tpeak (min)= 2.00 | 17.00
00407> Unit Hyd. peak (cms)= .59 | .07
00408> *#-----|-----|-----|-----|-----|-----|-----|-----|
00409> PEAK FLOW (cms)= .02 | .01 | *TOTALS*
00410> TIME TO PEAK (hrs)= 12.00 | 12.00 |
00411> RUNOFF VOLUME (mm)= 66.00 | 49.053 |
00412> TOTAL RAINFALL (mm)= 66.80 | 66.80 | 66.801 |
00413> RUNOFF COEFFICIENT = .99 | .48 | .734 |
00414>
00415> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00416> CN* = 79.0 | Ia = Dep. Storage (Above)
00417> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00418> THAN THE STORAGE COEFFICIENT.
00419> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00420>
00421>
00422> 002:0005-----|-----|-----|-----|-----|-----|-----|-----|
00423> *#-----|-----|-----|-----|-----|-----|-----|-----|
00424> *# Agricultural area draining to diversion swale at access road/solar module
00425> *#-----|-----|-----|-----|-----|-----|-----|-----|
00426>
00427> | DESIGN NASHYD | Area (ha)= 6.30 | Curve Number (CN)=79.00
00428> | ID3:202a DT= 5.00 | Ia (mm)= 1.500 | # of Linear Res.(N)= 3.00
00429> |-----|-----|-----|-----|-----|-----|-----|-----|
00430> | U.H. Tp(hrs)= .750 |
00431>
00432> Unit Hyd Qpeak (cms)= .321
00433>
00434> PEAK FLOW (cms)= .199 (i)
00435> TIME TO PEAK (hrs)= 12.667 | 18.00 |
00436> RUNOFF VOLUME (mm)= 32.105 |
00437> TOTAL RAINFALL (mm)= 66.801 |
00438> RUNOFF COEFFICIENT = .481 |
00439>
00440> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00441>
00442> 002:0006-----|-----|-----|-----|-----|-----|-----|-----|
00443> *#-----|-----|-----|-----|-----|-----|-----|-----|
00444> *# Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands
00445> *#-----|-----|-----|-----|-----|-----|-----|-----|
00446>
00447> | DESIGN STANDHYD | Area (ha)= 5.80
00448> | ID4:202b DT= 1.00 | Total Imp(%)= 50.00 | Dir. Conn.(%)= 50.00
00449> |-----|-----|-----|-----|-----|-----|-----|-----|
00450>
00451> | IMPERVIOUS | PERVIOUS (i)
00452> | Surface Area (ha)= 2.90 | 2.90
00453> | Dep. Storage (mm)= .80 | 1.50
00454> | Average Slope (%)= .90 | .90
00455> | Length (m)= 196.64 | 40.00
00456> | Mannings n = .013 | .250
00457>
00458> Max.eff.Inten.(mm/hr)= 73.75 | 35.90
00459> over (min) = 2.00 | 4.00
00460> Storage Coeff. (min)= 1.84 (ii) | 17.98 (ii)
00461> Unit Hyd. Tpeak (min)= 4.00 | 18.00
00462> Unit Hyd. peak (cms)= .26 | .06
00463>
00464> PEAK FLOW (cms)= .57 | .18 | *TOTALS*
00465> TIME TO PEAK (hrs)= 12.00 | 12.18 | 12.000 |
00466> RUNOFF VOLUME (mm)= 66.00 | 32.09 | 49.053 |
00467> TOTAL RAINFALL (mm)= 66.80 | 66.80 | 66.801 |
00468> RUNOFF COEFFICIENT = .99 | .48 | .734 |
00469>
00470> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00471> CN* = 79.0 | Ia = Dep. Storage (Above)
00472> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00473> THAN THE STORAGE COEFFICIENT.
00474> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00475>
00476> 002:0007-----|-----|-----|-----|-----|-----|-----|-----|
00477> *#-----|-----|-----|-----|-----|-----|-----|-----|
00478> *# Sum of flows to constructed wetland SWMF
00479> *#-----|-----|-----|-----|-----|-----|-----|-----|
00480>
00481> | ADD HYD (401 ) | ID: NHYD | AREA | QPEAK | TPEAK | R.V. | DWF
00482> |-----|-----|-----|-----|-----|-----|-----|
00483> | ID2 02:201b | (ha) | (cms) | (hrs) | (mm) | (cms) |
00484> | +ID2 04:202b | .20 | .025 | 12.00 | 49.05 | .000 |
00485> | +ID2 04:202b | 5.80 | .696 | 12.00 | 49.05 | .000 |
00486> |-----|-----|-----|-----|-----|-----|-----|
00487> | SUM 05:401 | 6.00 | .721 | 12.00 | 49.05 | .000 |
00488>
00489> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00490>
00491> 002:0008-----|-----|-----|-----|-----|-----|-----|-----|
00492> *#-----|-----|-----|-----|-----|-----|-----|-----|
00493> *# Constructed wetland SWMF
00494> *#-----|-----|-----|-----|-----|-----|-----|-----|
00495>
00496> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00497> | IN>05: (401 ) |
00498> | OUT<06: (DP1 ) |
00499>
00500> |-----|-----|-----|-----|-----|-----|-----|-----|
00501> | OFFFLOW STORAGE TABLE |
00502> | (cms) (ha.m.) | (cms) (ha.m.) |
00503> | .000 .0000E+00 | .121 .2361E+00 |
00504> | .003 .2890E-01 | .136 .2763E+00 |
00505> | .005 .5950E-01 | .150 .3180E+00 |
00506> | .016 .3160E-01 | .459 .3613E+00 |
00507> | .049 .1253E+00 | 1.087 .4063E+00 |
00508> | .081 .1607E+00 | 2.002 .4528E+00 |
00509> | .103 .1976E+00 | .000 .0000E+00 |
00510>
00511> ROUTING RESULTS | AREA | QPEAK | TPEAK | R.V.
00512> | (ha) | (cms) | (hrs) | (mm) |
00513> | INFLOW >05: (401 ) | 6.00 | .721 | 12.00 | 49.053 |
00514> | OUTFLOW<06: (DP1 ) | 6.00 | .087 | 13.000 | 49.050 |
00515>
00516> PEAK FLOW REDUCTION [Qout/Qin] (%)= 12.126
00517> TIME SHIFT OF PEAK FLOW (min)= 60.00
00518> MAXIMUM STORAGE USED (ha.m.)=.1716E+00
00519>
00520> 002:0009-----|-----|-----|-----|-----|-----|-----|-----|
00521> *#-----|-----|-----|-----|-----|-----|-----|-----|
00522> *# Solar module area draining to diversion swale at west side of module
00523> *#-----|-----|-----|-----|-----|-----|-----|-----|
00524>
00525> | DESIGN NASHYD | Area (ha)= 1.70 | Curve Number (CN)=79.00
00526> | ID7:202c DT= 5.00 | Ia (mm)= 1.500 | # of Linear Res.(N)= 3.00
00527> |-----|-----|-----|-----|-----|-----|-----|-----|
00528> | U.H. Tp(hrs)= .420 |
00529>
00530> Unit Hyd Qpeak (cms)= .155
00531>
00532> PEAK FLOW (cms)= .081 (i)
00533> TIME TO PEAK (hrs)= 12.250
00534> RUNOFF VOLUME (mm)= 32.105
00535> TOTAL RAINFALL (mm)= 66.801
00536> RUNOFF COEFFICIENT = .481
00537>
00538> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00539>
00540> 002:0010-----|-----|-----|-----|-----|-----|-----|-----|
00541> *#-----|-----|-----|-----|-----|-----|-----|-----|

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00541> | ADD HYD (woutSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00542> -----|-----|-----|-----|-----|-----|-----|
00543> | ID1 01:201a (ha) (cms) (hrs) (mm) (cms)
00544> | +ID2 02:201b .20 .025 12.00 49.05 .000
00545> | +ID3 03:202a 6.30 .199 12.67 32.11 .000
00546> | +ID4 04:202b 5.80 .696 12.00 49.05 .000
00547> | +ID5 07:202c 1.70 .081 12.25 32.11 .000
00548> -----|-----|-----|-----|-----|-----|
00549> | SUM 08:woutSW 24.90 .999 12.02 36.63 .000
00550>
00551> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00552>
00553> -----|-----|-----|-----|-----|-----|
00554> 002:0011-----|-----|-----|-----|-----|-----|
00555> *#-----|-----|-----|-----|-----|-----|
00556> -----|-----|-----|-----|-----|-----|
00557> | ADD HYD (withSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00558> -----|-----|-----|-----|-----|-----|
00559> | ID1 01:201a 10.90 .356 12.67 33.11 .000
00560> | +ID2 03:202a 6.30 .199 12.67 32.11 .000
00561> | +ID3 06:DP1 6.00 .087 13.00 49.05 .000
00562> | +ID4 07:202c 1.70 .081 12.25 32.11 .000
00563> -----|-----|-----|-----|-----|-----|
00564> | SUM 10:withSW 24.90 .702 12.58 36.63 .000
00565>
00566> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00567>
00568> -----|-----|-----|-----|-----|-----|
00569> 002:0012-----|-----|-----|-----|-----|-----|
00570> *#-----|-----|-----|-----|-----|-----|
00571> -----|-----|-----|-----|-----|-----|
00572> 002:0002-----|-----|-----|-----|-----|-----|
00573> ** END OF RUN : 2
00574>
00575> *****
00576>
00577>
00578>
00579>
00580>
00581>
00582> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00583> | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00584> | TZERO = .00 hrs on
00585> | METOUT= 2 (output = METRIC)
00586> | NRUN = 003
00587> | NSTORM= 1
00588> | # 1=si_10.STM
00589>
00590> 003:0002-----|-----|-----|-----|-----|-----|
00591> *#-----|-----|-----|-----|-----|-----|
00592> *# Project Name : Grand Renewable Energy Park - O & M Facility
00593> *# Project Number: 1610-10624
00594> *# Date : 1-27-2011
00595> *# Company : Stantec Consulting Ltd. (Kitchener)
00596> *# Modeller : S Robertson
00597> *# License # : 4730904
00598> *#-----|-----|-----|-----|-----|-----|
00599> *# PROPOSED CONDITIONS
00600> *****
00601> -----|-----|-----|-----|-----|-----|
00602> 003:0002-----|-----|-----|-----|-----|-----|
00603>
00604> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_10.STM
00605> | Ptotal= 77.40 mm | Comments: SCS-II 24H 10-YEAR SIMCOE
00606>
00607>
00608> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00609> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00610> .50 .851 | 6.50 1.548 | 12.50 11.146 | 18.50 1.393
00611> .75 .851 | 6.75 1.548 | 12.75 5.728 | 18.75 1.393
00612> 1.00 .851 | 7.00 1.548 | 13.00 5.728 | 19.00 1.393
00613> 1.25 .851 | 7.25 1.548 | 13.25 4.025 | 19.25 1.393
00614> 1.50 .851 | 7.50 1.548 | 13.50 4.334 | 19.50 1.393
00615> 1.75 .851 | 7.75 1.548 | 13.75 3.251 | 19.75 1.393
00616> 2.00 .851 | 8.00 1.548 | 14.00 3.251 | 20.00 1.393
00617> 2.25 1.006 | 8.25 2.090 | 14.25 2.322 | 20.25 .929
00618> 2.50 1.006 | 8.50 2.090 | 14.50 2.322 | 20.50 .929
00619> 2.75 1.006 | 8.75 2.090 | 14.75 2.322 | 20.75 .929
00620> 3.00 1.006 | 9.00 2.090 | 15.00 2.322 | 21.00 .929
00621> 3.25 1.006 | 9.25 2.477 | 15.25 2.322 | 21.25 .929
00622> 3.50 1.006 | 9.50 2.477 | 15.50 2.322 | 21.50 .929
00623> 3.75 1.006 | 9.75 2.786 | 15.75 2.322 | 21.75 .929
00624> 4.00 1.006 | 10.00 2.786 | 16.00 2.322 | 22.00 .929
00625> 4.25 1.238 | 10.25 3.560 | 16.25 1.393 | 22.25 .929
00626> 4.50 1.238 | 10.50 3.560 | 16.50 1.393 | 22.50 .929
00627> 4.75 1.238 | 10.75 4.799 | 16.75 1.393 | 22.75 .929
00628> 5.00 1.238 | 11.00 4.799 | 17.00 1.393 | 23.00 .929
00629> 5.25 1.238 | 11.25 4.430 | 17.25 1.393 | 23.25 .929
00630> 5.50 1.238 | 11.50 7.430 | 17.50 1.393 | 23.50 .929
00631> 5.75 1.238 | 11.75 32.198 | 17.75 1.393 | 23.75 .929
00632> 6.00 1.238 | 12.00 85.450 | 18.00 1.393 | 24.00 .929
00633>
00634>
00635> 003:0003-----|-----|-----|-----|-----|-----|
00636> *#-----|-----|-----|-----|-----|-----|
00637> *# Agricultural area to draining to diversion swale at east side of access road
00638> *#-----|-----|-----|-----|-----|-----|
00639> -----|-----|-----|-----|-----|-----|
00640> | DESIGN NASHYD | Area (ha)= 10.90 Curve Number (CN)=80.00
00641> | 01:201a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00642> | U.H. Tp(hrs)= .750
00643>
00644> Unit Hyd Qpeak (cms)= .555
00645>
00646> PEAK FLOW (cms)= .447 (i)
00647> TIME TO PEAK (hrs)= 12.667
00648> RUNOFF VOLUME (mm)= 41.324
00649> TOTAL RAINFALL (mm)= 77.398
00650> RUNOFF COEFFICIENT = .534
00651>
00652> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00653>
00654> -----|-----|-----|-----|-----|-----|
00655> 003:0004-----|-----|-----|-----|-----|-----|
00656> *#-----|-----|-----|-----|-----|-----|
00657> *# Access road right-of-way (most northerly 160 m stretch)
00658> *#-----|-----|-----|-----|-----|-----|
00659> -----|-----|-----|-----|-----|-----|
00660> | DESIGN STANDHYD | Area (ha)= .20
00661> | 02:201b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
00662>
00663>
00664> Surface Area (ha)= .10 IMPERVIOUS PERVIOUS (i)
00665> Dep. Storage (mm)= .80 1.50
00666> Average Slope (%)= .60 .60
00667> Length (m)= 36.51 40.00
00668> Mannings n = .013 .250
00669>
00670> Max.eff.Inten.(mm/hr)= 85.45 50.86
00671> over (min) 2.00 15.00
00672> Storage Coeff. (min)= 1.73 (ii) 15.01 (ii)
00673> Unit Hyd. Tpeak (min)= 2.00 15.00
00674> Unit Hyd. peak (cms)= .61 .08
00675>

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00676> PEAK FLOW (cms)= .02 .01 .030 (iii)
00677> TIME TO PEAK (hrs)= 12.00 12.13 12.000
00678> RUNOFF VOLUME (mm)= 76.59 40.16 58.383
00679> TOTAL RAINFALL (mm)= 77.40 77.40 77.399
00680> RUNOFF COEFFICIENT = .99 .52 .754
00681>
00682> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
00683> CN= 79.0 Ia = Dep. Storage (Above)
00684> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00685> THAN THE STORAGE COEFFICIENT.
00686> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00687>
00688> -----|-----|-----|-----|-----|-----|
00689> 003:0005-----|-----|-----|-----|-----|-----|
00690> *#-----|-----|-----|-----|-----|-----|
00691> *# Agricultural area draining to diversion swale at access road/solar module
00692> *#-----|-----|-----|-----|-----|-----|
00693> -----|-----|-----|-----|-----|-----|
00694> | DESIGN NASHYD | Area (ha)= 6.30 Curve Number (CN)=79.00
00695> | 03:202a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00696> | U.H. Tp(hrs)= .750
00697>
00698> Unit Hyd Qpeak (cms)= .321
00699>
00700> PEAK FLOW (cms)= .251 (i)
00701> TIME TO PEAK (hrs)= 12.667
00702> RUNOFF VOLUME (mm)= 40.166
00703> TOTAL RAINFALL (mm)= 77.398
00704> RUNOFF COEFFICIENT = .519
00705>
00706> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00707>
00708> -----|-----|-----|-----|-----|-----|
00709> 003:0006-----|-----|-----|-----|-----|-----|
00710> *#-----|-----|-----|-----|-----|-----|
00711> *# Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands
00712> *#-----|-----|-----|-----|-----|-----|
00713> -----|-----|-----|-----|-----|-----|
00714> | DESIGN STANDHYD | Area (ha)= 5.80
00715> | 04:202b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
00716>
00717> IMPERVIOUS PERVIOUS (i)
00718> Surface Area (ha)= 2.90 2.90
00719> Dep. Storage (mm)= .80 1.50
00720> Average Slope (%)= .90 .90
00721> Length (m)= 196.64 40.00
00722> Mannings n = .013 .250
00723>
00724> Max.eff.Inten.(mm/hr)= 85.45 48.69
00725> over (min) 4.00 16.00
00726> Storage Coeff. (min)= 4.21 (ii) 16.17 (ii)
00727> Unit Hyd. Tpeak (min)= 4.00 16.00
00728> Unit Hyd. peak (cms)= .27 .07
00729>
00730> PEAK FLOW (cms)= .67 .24 .847 (iii)
00731> TIME TO PEAK (hrs)= 12.00 12.15 12.000
00732> RUNOFF VOLUME (mm)= 76.60 40.16 58.383
00733> TOTAL RAINFALL (mm)= 77.40 77.40 77.399
00734> RUNOFF COEFFICIENT = .99 .52 .754
00735>
00736> (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
00737> CN= 79.0 Ia = Dep. Storage (Above)
00738> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00739> THAN THE STORAGE COEFFICIENT.
00740> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00741>
00742> -----|-----|-----|-----|-----|-----|
00743> *#-----|-----|-----|-----|-----|-----|
00744> *# Sum of flows to constructed wetland SWMF
00745> *#-----|-----|-----|-----|-----|-----|
00746> -----|-----|-----|-----|-----|-----|
00747> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00748> -----|-----|-----|-----|-----|-----|
00749> | ID1 02:201b .20 .030 12.00 58.38 .000
00750> | +ID2 04:202b 5.80 .847 12.00 58.38 .000
00751> -----|-----|-----|-----|-----|-----|
00752> | SUM 05:401 6.00 .878 12.00 58.38 .000
00753>
00754> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00755>
00756> -----|-----|-----|-----|-----|-----|
00757> 003:0007-----|-----|-----|-----|-----|-----|
00758> *#-----|-----|-----|-----|-----|-----|
00759> *# ROUTE RESERVOIR | Requested routing time step = 1.0 min.
00760> *#-----|-----|-----|-----|-----|-----|
00761> *# Constructed wetland SWMF
00762> *#-----|-----|-----|-----|-----|-----|
00763> | IN>05:(401 ) |
00764> | OUT<06:(DP1 ) |
00765> -----|-----|-----|-----|-----|-----|
00766> -----|-----|-----|-----|-----|-----|
00767> -----|-----|-----|-----|-----|-----|
00768> -----|-----|-----|-----|-----|-----|
00769> -----|-----|-----|-----|-----|-----|
00770> -----|-----|-----|-----|-----|-----|
00771> -----|-----|-----|-----|-----|-----|
00772> -----|-----|-----|-----|-----|-----|
00773> -----|-----|-----|-----|-----|-----|
00774> -----|-----|-----|-----|-----|-----|
00775> -----|-----|-----|-----|-----|-----|
00776> -----|-----|-----|-----|-----|-----|
00777> -----|-----|-----|-----|-----|-----|
00778> -----|-----|-----|-----|-----|-----|
00779> -----|-----|-----|-----|-----|-----|
00780> -----|-----|-----|-----|-----|-----|
00781> -----|-----|-----|-----|-----|-----|
00782> -----|-----|-----|-----|-----|-----|
00783> -----|-----|-----|-----|-----|-----|
00784> -----|-----|-----|-----|-----|-----|
00785> -----|-----|-----|-----|-----|-----|
00786> 003:0009-----|-----|-----|-----|-----|-----|
00787> *# Solar module area draining to diversion swale at west side of module
00788> *#-----|-----|-----|-----|-----|-----|
00789> -----|-----|-----|-----|-----|-----|
00790> | DESIGN NASHYD | Area (ha)= 1.70 Curve Number (CN)=79.00
00791> | 07:202c DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
00792> | U.H. Tp(hrs)= 1.420
00793>
00794> Unit Hyd Qpeak (cms)= .155
00795>
00796> PEAK FLOW (cms)= .102 (i)
00797> TIME TO PEAK (hrs)= 12.250
00798> RUNOFF VOLUME (mm)= 40.166
00799> TOTAL RAINFALL (mm)= 77.398
00800> RUNOFF COEFFICIENT = .519
00801>
00802> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00803>
00804> -----|-----|-----|-----|-----|-----|
00805> 003:0010-----|-----|-----|-----|-----|-----|
00806> *#-----|-----|-----|-----|-----|-----|
00807> -----|-----|-----|-----|-----|-----|
00808> | ADD HYD (woutSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00809> -----|-----|-----|-----|-----|-----|
00810> | ID1 01:201a 10.90 .447 12.67 41.32 .000

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00811> +ID2 02:201b .20 .030 12.00 58.38 .000
00812> +ID3 03:202a 6.30 .251 12.67 40.17 .000
00813> +ID4 04:202b 5.80 .106 12.00 58.38 .000
00814> +ID5 07:202c 1.70 .102 12.25 40.17 .000
00815> -----
00816> SUM 08:woutSW 24.90 1.234 12.02 45.06 .000
00817>
00818> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00819>
00820> -----
00821> 003:0011-----
00822> *#-----
00823> -----
00824> | ADD HYD (withSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
00825> |-----|-----|-----|-----|-----|-----|
00826> | | | (ha) (cms) (hrs) (mm) (cms)
00827> | ID1 01:201a 10.90 .447 12.67 41.32 .000
00828> | +ID2 03:202a 6.30 .251 12.67 40.17 .000
00829> | +ID3 06:DP1 6.00 .106 12.92 58.38 .000
00830> | +ID4 07:202c 1.70 .102 12.25 40.17 .000
00831> |-----|-----|-----|-----|-----|-----|
00832> | SUM 10:withSW 24.90 .879 12.58 45.06 .000
00833>
00834> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
00835>
00836> 003:0012-----
00837> *#-----
00838> -----
00839> 003:0002-----
00840>
00841> 003:0002-----
00842> ** END OF RUN : 3
00843>
00844> -----
00845> -----
00846> -----
00847> -----
00848> -----
00849> -----
00850> -----
00851> | START | Project dir.: C:\PROGRA-1\SWMHYMO\
00852> | | Rainfall dir.: C:\PROGRA-1\SWMHYMO\
00853> | | ZERO on 0
00854> | METOUT= 2 (output = METRIC)
00855> | NRUN = 004
00856> | NSTORM= 1
00857> | # 1=si_25.STM
00858>
00859> 004:0002-----
00860> *#-----
00861> *# Project Name : Grand Renewable Energy Park - O & M Facility
00862> *# Project Number: 1610-10624
00863> *# Date : 1-27-2011
00864> *# Company : Stantec Consulting Ltd. (Kitchener)
00865> *# Modeller : S Robertson
00866> *# License # : 4730904
00867> *#-----
00868> *# PROPOSED CONDITIONS
00869> *#-----
00870>
00871> 004:0002-----
00872>
00873> | READ STORM | Filename: C:\PROGRA-1\SWMHYMO\si_25.STM
00874> | Ptotal= 90.80 mm | Comments: SCS-II 24H 25-YEAR SIMCOE
00875> -----
00876> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
00877> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
00878> .25 .999 | 6.5 1.816 | 12.25 13.075 | 18.25 1.634
00879> .50 .999 | 6.50 1.816 | 12.50 13.075 | 18.50 1.634
00880> .75 .999 | 6.75 1.816 | 12.75 6.719 | 18.75 1.634
00881> 1.00 .999 | 7.00 1.816 | 13.00 6.719 | 19.00 1.634
00882> 1.25 .999 | 7.25 1.816 | 13.25 4.722 | 19.25 1.634
00883> 1.50 .999 | 7.50 1.816 | 13.50 5.085 | 19.50 1.634
00884> 1.75 .999 | 7.75 1.816 | 13.75 3.814 | 19.75 1.634
00885> 2.00 .999 | 8.00 1.816 | 14.00 3.814 | 20.00 1.634
00886> 2.25 1.180 | 8.25 2.452 | 14.25 2.724 | 20.25 1.090
00887> 2.50 1.180 | 8.50 2.452 | 14.50 2.724 | 20.50 1.090
00888> 2.75 1.180 | 8.75 2.452 | 14.75 2.724 | 20.75 1.090
00889> 3.00 1.180 | 9.00 2.452 | 15.00 2.724 | 21.00 1.090
00890> 3.25 1.180 | 9.25 2.906 | 15.25 2.724 | 21.25 1.090
00891> 3.50 1.180 | 9.50 2.906 | 15.50 2.724 | 21.50 1.090
00892> 3.75 1.180 | 9.75 3.269 | 15.75 2.724 | 21.75 1.090
00893> 4.00 1.180 | 10.00 3.269 | 16.00 2.724 | 22.00 1.090
00894> 4.25 1.453 | 10.25 1.177 | 16.25 1.634 | 22.25 1.090
00895> 4.50 1.453 | 10.50 4.177 | 16.50 1.634 | 22.50 1.090
00896> 4.75 1.453 | 10.75 5.630 | 16.75 1.634 | 22.75 1.090
00897> 5.00 1.453 | 11.00 5.630 | 17.00 1.634 | 23.00 1.090
00898> 5.25 1.453 | 11.25 8.717 | 17.25 1.634 | 23.25 1.090
00899> 5.50 1.453 | 11.50 8.717 | 17.50 1.634 | 23.50 1.090
00900> 5.75 1.453 | 11.75 37.773 | 17.75 1.634 | 23.75 1.090
00901> 6.00 1.453 | 12.00 100.243 | 18.00 1.634 | 24.00 1.090
00902>
00903> -----
00904> 004:0003-----
00905> *#-----
00906> *# Agricultural area to draining to diversion swale at east side of access road
00907> *#-----
00908> -----
00909> | DESIGN NASHYD | Area (ha)= 10.90 Curve Number (CN)=80.00
00910> | 01:201a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00911> | U.H. Tp(hrs)= .750
00912>
00913> Unit Hyd Qpeak (cms)= .555
00914>
00915> PEAK FLOW (cms)= .567 (i)
00916> TIME TO PEAK (hrs)= 12.667
00917> RUNOFF VOLUME (mm)= 52.190
00918> TOTAL RAINFALL (mm)= 90.801
00919> RUNOFF COEFFICIENT = .575
00920>
00921> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00922>
00923> -----
00924> 004:0004-----
00925> *#-----
00926> *# Access road right-of-way (most northerly 160 m stretch)
00927> *#-----
00928> -----
00929> | DESIGN STANDHYD | Area (ha)= .20
00930> | 02:201b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
00931>
00932> IMPERVIOUS PERVIOUS (i)
00933> Surface Area (ha)= .10
00934> Dep. Storage (mm)= .80 1.50
00935> Average Slope (%)= .60 .60
00936> Length (m)= 36.51 40.00
00937> Mannings n = .013 .250
00938>
00939> Max.eff.Inten.(mm/hr)= 100.24 65.03
00940> over (min) 2.00 14.00
00941> Storage Coeff. (min)= 1.63 (ii) 13.66 (ii)
00942> Unit Hyd. Tpeak (min)= 2.00 14.00
00943> Unit Hyd. peak (cms)= .63 .08
00944>
00945> PEAK FLOW (cms)= .03 .01 *TOTALS*
00946> .037 (iii)

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00946> TIME TO PEAK (hrs)= 12.00 12.12 12.000
00947> RUNOFF VOLUME (mm)= 90.00 50.85 70.427
00948> TOTAL RAINFALL (mm)= 90.80 90.80 90.802
00949> RUNOFF COEFFICIENT = .99 .56 .776
00950>
00951> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
00952> CN* = 79.0 Ia = Dep. Storage (Above)
00953> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
00954> THAN THE STORAGE COEFFICIENT.
00955> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00956>
00957> -----
00958> -----
00959> *#-----
00960> *# Agricultural area draining to diversion swale at access road/solar module
00961> *#-----
00962> -----
00963> | DESIGN NASHYD | Area (ha)= 6.30 Curve Number (CN)=79.00
00964> | 03:202a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
00965> | U.H. Tp(hrs)= .750
00966>
00967> Unit Hyd Qpeak (cms)= .321
00968>
00969> PEAK FLOW (cms)= .319 (i)
00970> TIME TO PEAK (hrs)= 12.667
00971> RUNOFF VOLUME (mm)= 50.852
00972> TOTAL RAINFALL (mm)= 90.801
00973> RUNOFF COEFFICIENT = .560
00974>
00975> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
00976>
00977> -----
00978> -----
00979> *#-----
00980> *# Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands
00981> *#-----
00982> -----
00983> | DESIGN STANDHYD | Area (ha)= 5.80
00984> | 04:202b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
00985>
00986> IMPERVIOUS PERVIOUS (i)
00987> Surface Area (ha)= 2.90 2.90
00988> Dep. Storage (mm)= .80 1.50
00989> Average Slope (%)= .90 .90
00990> Length (m)= 196.64 40.00
00991> Mannings n = .013 .250
00992>
00993> Max.eff.Inten.(mm/hr)= 100.24 64.45
00994> over (min) 4.00 15.00
00995> Storage Coeff. (min)= 3.95 (ii) 14.64 (ii)
00996> Unit Hyd. Tpeak (min)= 4.00 15.00
00997> Unit Hyd. peak (cms)= .28 .08
00998>
00999> PEAK FLOW (cms)= .79 .32 *TOTALS*
01000> TIME TO PEAK (hrs)= 12.00 12.13 12.000
01001> RUNOFF VOLUME (mm)= 89.99 50.84 70.427
01002> TOTAL RAINFALL (mm)= 90.80 90.80 90.802
01003> RUNOFF COEFFICIENT = .99 .56 .776
01004>
01005> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01006> CN* = 79.0 Ia = Dep. Storage (Above)
01007> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01008> THAN THE STORAGE COEFFICIENT.
01009> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01010>
01011> -----
01012> 004:0007-----
01013> *#-----
01014> *# Sum of flows to constructed wetland SWMF
01015> *#-----
01016> -----
01017> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01018> |-----|-----|-----|-----|-----|-----|
01019> | ID1 02:201b .20 .037 12.00 70.43 .000
01020> | +ID2 04:202b 5.80 1.041 12.00 70.43 .000
01021> |-----|-----|-----|-----|-----|-----|
01022> | SUM 05:401 6.00 1.078 12.00 70.43 .000
01023>
01024> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01025>
01026> -----
01027> 004:0008-----
01028> *#-----
01029> *# Constructed wetland SWMF
01030> *#-----
01031> -----
01032> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01033> | IN:05:401 ) |
01034> | OUT:06:DP1 ) |
01035> -----
01036> |-----|-----|-----|-----|-----|-----|
01037> | | | | |
01038> | (cms) (ha.m.) | (cms) (ha.m.) |
01039> | .000 .0000E+00 | .121 .2361E+00
01040> | .003 .2890E-01 | .136 .2763E+00
01041> | .005 .5950E-01 | .150 .3180E+00
01042> | .016 .9160E-01 | .459 .3613E+00
01043> | .049 .1253E+00 | 1.087 .4063E+00
01044> | .081 .1607E+00 | 2.002 .4528E+00
01045> | .103 .1976E+00 | .000 .0000E+00
01046>
01047> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01048> (ha) (cms) (hrs) (mm)
01049> INFLOW>05: (401 ) 6.00 1.078 12.00 70.427
01050> OUTFLOW<06: (DP1 ) 6.00 .124 12.900 70.423
01051>
01052> PEAK FLOW REDUCTION [Qout/Qin] (%)= 11.537
01053> TIME SHIFT OF PEAK FLOW (min)= 54.00
01054> MAXIMUM STORAGE USED (ha.m.)=.2451E+00
01055>
01056> 004:0009-----
01057> *#-----
01058> -----
01059> | DESIGN NASHYD | Area (ha)= 1.70 Curve Number (CN)=79.00
01060> | 07:202c DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01061> | U.H. Tp(hrs)= .420
01062>
01063> Unit Hyd Qpeak (cms)= .155
01064>
01065> PEAK FLOW (cms)= .130 (i)
01066> TIME TO PEAK (hrs)= 12.250
01067> RUNOFF VOLUME (mm)= 50.852
01068> TOTAL RAINFALL (mm)= 90.801
01069> RUNOFF COEFFICIENT = .560
01070>
01071> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01072>
01073> -----
01074> 004:0010-----
01075> *#-----
01076> -----
01077> | ADD HYD (woutSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01078> |-----|-----|-----|-----|-----|-----|
01079> | ID1 01:201a 10.90 .567 12.67 52.19 .000
01080> | +ID2 02:201b .20 .037 12.00 70.43 .000

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01081> +ID3 03:202a 6.30 .319 12.67 50.85 .000
01082> +ID4 04:202b 5.80 1.041 12.00 70.43 .000
01083> +ID5 07:202c 1.70 .130 12.25 50.85 .000
01084> -----
01085> SUM 08:woutSW 24.90 1.539 12.02 56.16 .000
01086>
01087> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01088>
01089>
01090> 004:0011-----
01091> #*-----
01092> | ADD HYD (withSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01093> | (ha) (cms) (hrs) (mm) (cms)
01094> ID1 01:201a 10.90 .567 12.67 52.19 .000
01095> +ID2 03:202a 6.30 .319 12.67 50.85 .000
01096> +ID3 06:DP1 6.00 .124 12.90 70.42 .000
01097> +ID4 07:202c 1.70 .130 12.25 50.85 .000
01098> -----
01099> SUM 10:withSW 24.90 1.108 12.58 56.15 .000
01100>
01101> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01102>
01103>
01104>
01105> 004:0012-----
01106> #*-----
01107> |-----
01108> 004:0002-----
01109> #*-----
01110> 004:0002-----
01111> #*-----
01112> 004:0002-----
01113> ** END OF RUN : 4
01114>
01115> *****
01116>
01117>
01118>
01119>
01120>
01121>
01122> | START | Project dir.: C:\PROGRA-1\SWMHYM\
01123> | Rainfall dir.: C:\PROGRA-1\SWMHYM\
01124> TZERO = .00 hrs on 0
01125> METOUT= 2 (output = METRIC)
01126> NRUN = 005
01127> NSTORM= 1
01128> # 1=si_50.STM
01129>
01130> 005:0002-----
01131> #*-----
01132> # Project Name : Grand Renewable Energy Park - O & M Facility
01133> # Project Number: 1610-10624
01134> # Date : 1-27-2011
01135> # Company : Stantec Consulting Ltd. (Kitchener)
01136> # Modeller : S Robertson
01137> # License # : 4730904
01138> #*****
01139> # PROPOSED CONDITIONS
01140> #*****
01141>
01142> 005:0002-----
01143> #*-----
01144> | READ STORM | Filename: C:\PROGRA-1\SWMHYM\si_50.STM
01145> | Ptotal= 100.70 mm | Comments: SCS-II 24H 50-YEAR SIMCOE
01146>
01147> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01148> hr mm/hr | hr mm/hr | hr mm/hr | hr mm/hr
01149> .25 1.108 | 6.25 2.014 | 12.25 4.501 | 18.25 1.813
01150> .50 1.108 | 6.50 2.014 | 12.50 4.501 | 18.50 1.813
01151> .75 1.108 | 6.75 2.014 | 12.75 7.452 | 18.75 1.813
01152> 1.00 1.108 | 7.00 2.014 | 13.00 7.452 | 19.00 1.813
01153> 1.25 1.108 | 7.25 2.014 | 13.25 5.236 | 19.25 1.813
01154> 1.50 1.108 | 7.50 2.014 | 13.50 5.639 | 19.50 1.813
01155> 1.75 1.108 | 7.75 2.014 | 13.75 4.229 | 19.75 1.813
01156> 2.00 1.108 | 8.00 2.014 | 14.00 4.229 | 20.00 1.813
01157> 2.25 1.309 | 8.25 2.719 | 14.25 3.021 | 20.25 1.208
01158> 2.50 1.309 | 8.50 2.719 | 14.50 3.021 | 20.50 1.208
01159> 2.75 1.309 | 8.75 2.719 | 14.75 3.021 | 20.75 1.208
01160> 3.00 1.309 | 9.00 2.719 | 15.00 3.021 | 21.00 1.208
01161> 3.25 1.309 | 9.25 3.222 | 15.25 3.021 | 21.25 1.208
01162> 3.50 1.309 | 9.50 3.222 | 15.50 3.021 | 21.50 1.208
01163> 3.75 1.309 | 9.75 3.625 | 15.75 3.021 | 21.75 1.208
01164> 4.00 1.309 | 10.00 3.625 | 16.00 3.021 | 22.00 1.208
01165> 4.25 1.611 | 10.25 4.632 | 16.25 1.813 | 22.25 1.208
01166> 4.50 1.611 | 10.50 4.632 | 16.50 1.813 | 22.50 1.208
01167> 4.75 1.611 | 10.75 6.243 | 16.75 1.813 | 22.75 1.208
01168> 5.00 1.611 | 11.00 6.243 | 17.00 1.813 | 23.00 1.208
01169> 5.25 1.611 | 11.25 9.667 | 17.25 1.813 | 23.25 1.208
01170> 5.50 1.611 | 11.50 9.667 | 17.50 1.813 | 23.50 1.208
01171> 5.75 1.611 | 11.75 41.891 | 17.75 1.813 | 23.75 1.208
01172> 6.00 1.611 | 12.00 111.173 | 18.00 1.813 | 24.00 1.208
01173>
01174>
01175> 005:0003-----
01176> #*-----
01177> # Agricultural area to draining to diversion swale at east side of access road
01178> #*-----
01179>
01180> | DESIGN NASHYD | Area (ha)= 10.90 Curve Number (CN)=80.00
01181> | 01:201a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
01182> | U.H. Tp(hrs)= .750
01183>
01184> Unit Hyd Qpeak (cms)= .555
01185>
01186> PEAK FLOW (cms)= .659 (i)
01187> TIME TO PEAK (hrs)= 12.667
01188> RUNOFF VOLUME (mm)= 60.483
01189> TOTAL RAINFALL (mm)= 100.699
01190> RUNOFF COEFFICIENT = .601
01191>
01192> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01193>
01194>
01195> 005:0004-----
01196> #*-----
01197> # Access road right-of-way (most northerly 160 m stretch)
01198> #*-----
01199>
02000> | DESIGN STANDHYD | Area (ha)= .20
02001> | 02:201b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
02002>
02003> IMPERVIOUS PERVIOUS (i)
02004> Surface Area (ha)= .10 .10
02005> Dep. Storage (mm)= .80 1.50
02006> Average Slope (%)= .60 .60
02007> Length (m)= 36.51 40.00
02008> Mannings n = .013 .250
02009>
02010> Max.eff.Inten.(mm/hr)= 111.17 76.06
02011> over (min) 2.00 13.00
02012> Storage Coeff. (min)= 1.56 (ii) 12.86 (ii)
02013> Unit Hyd. Tpeak (min)= 2.00 13.00
02014> Unit Hyd. peak (cms)= .65 .09
02015>
02016> *TOTALS*

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01216> PEAK FLOW (cms)= .03 .01 .043 (iii)
01217> TIME TO PEAK (hrs)= 12.00 12.10 12.000
01218> RUNOFF VOLUME (mm)= 59.90 59.01 79.463
01219> TOTAL RAINFALL (mm)= 100.70 100.70 100.700
01220> RUNOFF COEFFICIENT = .99 .59 .789
01221>
01222> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01223> CN = 79.0 Ia = Dep. Storage (Above)
01224> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01225> THAN THE STORAGE COEFFICIENT.
01226> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01227>
01228>
01229> 005:0005-----
01230> #*-----
01231> # Agricultural area draining to diversion swale at access road/solar module
01232> #*-----
01233>
01234> | DESIGN NASHYD | Area (ha)= 6.30 Curve Number (CN)=79.00
01235> | 03:202a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
01236> | U.H. Tp(hrs)= .750
01237>
01238> Unit Hyd Qpeak (cms)= .321
01239>
01240> PEAK FLOW (cms)= .371 (i)
01241> TIME TO PEAK (hrs)= 12.667
01242> RUNOFF VOLUME (mm)= 59.025
01243> TOTAL RAINFALL (mm)= 100.699
01244> RUNOFF COEFFICIENT = .586
01245>
01246> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01247>
01248>
01249> 005:0006-----
01250> #*-----
01251> # Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands
01252> #*-----
01253>
01254> | DESIGN STANDHYD | Area (ha)= 5.80
01255> | 04:202b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
01256>
01257> IMPERVIOUS PERVIOUS (i)
01258> Surface Area (ha)= 2.90 2.90
01259> Dep. Storage (mm)= .80 1.50
01260> Average Slope (%)= .90 .90
01261> Length (m)= 196.64 40.00
01262> Mannings n = .013 .250
01263>
01264> Max.eff.Inten.(mm/hr)= 111.17 75.45
01265> over (min) 4.00 14.00
01266> Storage Coeff. (min)= 3.79 (ii) 13.83 (ii)
01267> Unit Hyd. Tpeak (min)= 4.00 14.00
01268> Unit Hyd. peak (cms)= .29 .08
01269>
01270> PEAK FLOW (cms)= .88 .39 1.194 (iii)
01271> TIME TO PEAK (hrs)= 12.00 12.12 12.000
01272> RUNOFF VOLUME (mm)= 99.89 59.01 79.463
01273> TOTAL RAINFALL (mm)= 100.70 100.70 100.700
01274> RUNOFF COEFFICIENT = .99 .59 .789
01275>
01276> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01277> CN = 79.0 Ia = Dep. Storage (Above)
01278> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01279> THAN THE STORAGE COEFFICIENT.
01280> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01281>
01282>
01283>
01284> #*-----
01285> # Sum of flows to constructed wetland SWMF
01286> #*-----
01287>
01288> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01289> | (ha) (cms) (hrs) (mm) (cms)
01290> ID1 02:201b .20 .043 12.00 79.46 .000
01291> +ID2 04:202b 5.80 .194 12.00 79.46 .000
01292> -----
01293> SUM 05:401 6.00 1.236 12.00 79.46 .000
01294>
01295> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01296>
01297>
01298>
01299> #*-----
01300> # Constructed wetland SWMF
01301> #*-----
01302>
01303> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01304> | IN>05:(401 ) |
01305> | OUT<06:(DP1 ) |
01306>
01307> ===== OUTFLOW STORAGE TABLE =====
01308> OUTFLOW STORAGE | OUTFLOW STORAGE
01309> (cms) (ha.m.) | (cms) (ha.m.)
01310> .000 .000E+00 | .121 .2361E+00
01311> .003 .2890E-01 | .136 .2763E+00
01312> .005 .5950E-01 | .150 .3180E+00
01313> .016 .9160E-01 | .459 .3613E+00
01314> .049 .1253E+00 | 1.087 .4063E+00
01315> .081 .1607E+00 | 2.002 .4528E+00
01316> .103 .1976E+00 | .000 .0000E+00
01317>
01318> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01319> (ha) (cms) (hrs) (mm)
01320> INFLOW>05: (401 ) 6.00 1.236 12.000 79.463
01321> OUTFLOW<06: (DP1 ) 6.00 .136 12.900 79.459
01322>
01323> PEAK FLOW REDUCTION [Qout/Qin] (%) = 11.024
01324> TIME SHIFT OF PEAK FLOW (min)= 54.00
01325> MAXIMUM STORAGE USED (ha.m.)= 2.771E+00
01326>
01327> 005:0009-----
01328> #* Solar module area draining to diversion swale at west side of module
01329> #*-----
01330>
01331> | DESIGN NASHYD | Area (ha)= 1.70 Curve Number (CN)=79.00
01332> | 07:202c DT= 5.00 | Ia (mm)= 1.500 # of Linear Res. (N)= 3.00
01333> | U.H. Tp(hrs)= .420
01334>
01335> Unit Hyd Qpeak (cms)= .155
01336>
01337> PEAK FLOW (cms)= .151 (i)
01338> TIME TO PEAK (hrs)= 12.250
01339> RUNOFF VOLUME (mm)= 59.024
01340> TOTAL RAINFALL (mm)= 100.699
01341> RUNOFF COEFFICIENT = .586
01342>
01343> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01344>
01345> 005:0010-----
01346> #*-----
01347>
01348> | ADD HYD (woutSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01349> | (ha) (cms) (hrs) (mm) (cms)
01350> ID1 01:201a 10.90 .659 12.67 60.48 .000

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01351> +ID2 02:201b .20 .043 12.00 79.46 .000
01352> +ID3 03:202a 6.30 .371 12.67 59.02 .000
01353> +ID4 04:202b 5.80 1.194 12.00 79.46 .000
01354> +ID5 07:202c 1.70 .151 12.25 59.02 .000
01355> -----
01356> SUM 08:woutSW 24.90 1.777 12.02 64.59 .000
01357>
01358> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01359>
01360> -----
01361> 005:0011-----
01362> *#-----
01363>
01364> | ADD HYD (withSW) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01365> |-----|-----|-----|-----|-----|-----|
01366> | | | (ha) (cms) (hrs) (mm) (cms)
01367> | ID1 01:201a 10.90 .659 12.67 60.48 .000
01368> | +ID2 03:202a 6.30 .371 12.67 59.02 .000
01369> | +ID3 03:202b 5.80 1.194 12.00 79.46 .000
01370> | +ID4 07:202c 1.70 .151 12.25 59.02 .000
01371> |-----|-----|-----|-----|-----|-----|
01372> | SUM 10:withSW 24.90 1.280 12.58 64.58 .000
01373>
01374> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01375>
01376> 005:0012-----
01377> *#-----
01378>
01379> 005:0002-----
01380>
01381> 005:0002-----
01382>
01383> 005:0002-----
01384>
01385> 005:0002-----
01386> ** END OF RUN : 5
01387>
01388> -----
01389>
01390>
01391>
01392>
01393>
01394>
01395> | START | Project dir.: C:\PROGRA-1\SWMHYM\
01396> | Rainfall dir.: C:\PROGRA-1\SWMHYM\
01397> | TZERO = .00 hrs on 0
01398> | METOUT= 2 (output = METRIC)
01399> | NRUN = 006
01400> | NSTORM= 1
01401> | # 1=si_100.STM
01402>
01403> 006:0002-----
01404> *#-----
01405> *# Project Name : Grand Renewable Energy Park - O & M Facility
01406> *# Project Number: 1610-10624
01407> *# Date : 1-27-2011
01408> *# Company : Stantec Consulting Ltd. (Kitchener)
01409> *# Modeller : S Robertson
01410> *# License # : 4730904
01411> *#-----
01412> *# PROPOSED CONDITIONS
01413> *#-----
01414>
01415> 006:0002-----
01416>
01417> | READ STORM | Filename: C:\PROGRA-1\SWMHYM\si_100.STM
01418> | Ptotal= 110.60 mm | Comments: SCS-II 24H 100-YEAR SIMCOE
01419>
01420> TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
01421> hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
01422> .25 1.217 | 6.25 2.212 | 12.25 15.926 | 18.25 1.991
01423> 1.75 1.217 | 7.75 2.212 | 13.75 4.645 | 19.75 1.991
01424> .75 1.217 | 6.75 2.212 | 12.75 8.184 | 18.75 1.991
01425> 1.00 1.217 | 7.00 2.212 | 13.00 8.184 | 19.00 1.991
01426> 1.25 1.217 | 7.25 2.212 | 13.25 5.751 | 19.25 1.991
01427> 1.50 1.217 | 7.50 2.212 | 13.50 6.194 | 19.50 1.991
01428> 1.75 1.217 | 7.75 2.212 | 13.75 4.645 | 19.75 1.991
01429> 2.00 1.217 | 8.00 2.212 | 14.00 4.645 | 20.00 1.991
01430> 2.25 1.438 | 8.25 2.986 | 14.25 3.318 | 20.25 1.327
01431> 2.50 1.438 | 8.50 2.986 | 14.50 3.318 | 20.50 1.327
01432> 2.75 1.438 | 8.75 2.986 | 14.75 3.318 | 20.75 1.327
01433> 3.00 1.438 | 9.00 2.986 | 15.00 3.318 | 21.00 1.327
01434> 3.25 1.438 | 9.25 2.986 | 15.25 3.318 | 21.25 1.327
01435> 3.50 1.438 | 9.50 3.539 | 15.50 3.318 | 21.50 1.327
01436> 3.75 1.438 | 9.75 3.982 | 15.75 3.318 | 21.75 1.327
01437> 4.00 1.438 | 10.00 3.982 | 16.00 3.318 | 22.00 1.327
01438> 4.25 1.770 | 10.25 5.088 | 16.25 1.991 | 22.25 1.327
01439> 4.50 1.770 | 10.50 5.088 | 16.50 1.991 | 22.50 1.327
01440> 4.75 1.770 | 10.75 6.857 | 16.75 1.991 | 22.75 1.327
01441> 5.00 1.770 | 11.00 6.857 | 17.00 1.991 | 23.00 1.327
01442> 5.25 1.770 | 11.25 10.618 | 17.25 1.991 | 23.25 1.327
01443> 5.50 1.770 | 11.50 10.618 | 17.50 1.991 | 23.50 1.327
01444> 5.75 1.770 | 11.75 46.010 | 17.75 1.991 | 23.75 1.327
01445> 6.00 1.770 | 12.00 122.102 | 18.00 1.991 | 24.00 1.327
01446>
01447>
01448> 006:0004-----
01449> *#-----
01450> *# Agricultural area to draining to diversion swale at east side of access road
01451> *#-----
01452>
01453> | DESIGN NASHYD | Area (ha)= 10.90 Curve Number (CN)=80.00
01454> | ID1:201a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01455> | U.H. Tp(hrs)= .750
01456>
01457> Unit Hyd Qpeak (cms)= .555
01458>
01459> PEAK FLOW (cms)= .753 (i)
01460> TIME TO PEAK (hrs)= 12.667
01461> RUNOFF VOLUME (mm)= 68.963
01462> TOTAL RAINFALL (mm)= 110.602
01463> RUNOFF COEFFICIENT = .624
01464>
01465> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01466>
01467> -----
01468>
01469> *#-----
01470> *# Access road right-of-way (most northerly 160 m stretch)
01471> *#-----
01472>
01473> | DESIGN STANDHYD | Area (ha)= .20
01474> | ID2:201b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
01475>
01476> IMPERVIOUS PERVIOUS (i)
01477> Surface Area (ha)= .10 .10
01478> Dep. Storage (mm)= .80 1.50
01479> Average Slope (%)= .60 .60
01480> Length (m)= 36.51 40.00
01481> Mannings n = .013 .250
01482>
01483> Max. eff. Inten. (mm/hr)= 122.10 87.34
01484> over (min) = 2.00 12.00
01485> Storage Coeff. (min)= 1.50 (ii) 12.19 (ii)

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01486> Unit Hyd. Tpeak (min)= 2.00 12.00
01487> Unit Hyd. peak (cms)= .66 .09
01488> *TOTALS*
01489> PEAK FLOW (cms)= .03 .02 .048 (iii)
01490> TIME TO PEAK (hrs)= 12.00 12.08 12.000
01491> RUNOFF VOLUME (mm)= 109.80 67.38 88.599
01492> TOTAL RAINFALL (mm)= 110.60 110.60 110.603
01493> RUNOFF COEFFICIENT = .99 .61 .801
01494>
01495> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01496> CN* = 79.0 Ia = Dep. Storage (Above)
01497> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01498> THAN THE STORAGE COEFFICIENT.
01499> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01500>
01501> -----
01502> 006:0005-----
01503> *#-----
01504> *# Agricultural area draining to diversion swale at access road/solar module
01505> *#-----
01506>
01507> | DESIGN NASHYD | Area (ha)= 6.30 Curve Number (CN)=79.00
01508> | ID3:202a DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01509> | U.H. Tp(hrs)= .750
01510>
01511> Unit Hyd Qpeak (cms)= .321
01512>
01513> PEAK FLOW (cms)= .424 (i)
01514> TIME TO PEAK (hrs)= 12.667
01515> RUNOFF VOLUME (mm)= 67.394
01516> TOTAL RAINFALL (mm)= 110.602
01517> RUNOFF COEFFICIENT = .609
01518>
01519> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01520>
01521> -----
01522> 006:0006-----
01523> *#-----
01524> *# Southern 2/3 of access road, O&M facility, SWMF, and surrounding lands
01525> *#-----
01526>
01527> | DESIGN STANDHYD | Area (ha)= 5.80
01528> | ID4:202b DT= 1.00 | Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
01529>
01530> IMPERVIOUS PERVIOUS (i)
01531> Surface Area (ha)= 2.90 2.90
01532> Dep. Storage (mm)= .80 1.50
01533> Average Slope (%)= .90 .90
01534> Length (m)= 196.64 40.00
01535> Mannings n = .013 .250
01536>
01537> Max. eff. Inten. (mm/hr)= 122.10 86.72
01538> over (min) = 4.00 13.00
01539> Storage Coeff (min)= 3.65 (ii) 13.15 (ii)
01540> Unit Hyd. Tpeak (min)= 4.00 13.00
01541> Unit Hyd. peak (cms)= .30 .09
01542>
01543> PEAK FLOW (cms)= .97 .46 *TOTALS* (iii)
01544> TIME TO PEAK (hrs)= 12.00 12.10 12.000
01545> RUNOFF VOLUME (mm)= 109.80 67.38 88.599
01546> TOTAL RAINFALL (mm)= 110.60 110.60 110.603
01547> RUNOFF COEFFICIENT = .99 .61 .801
01548>
01549> (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
01550> CN* = 79.0 Ia = Dep. Storage (Above)
01551> (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
01552> THAN THE STORAGE COEFFICIENT.
01553> (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01554>
01555> -----
01556> 006:0007-----
01557> *#-----
01558> *# Sum of flows to constructed wetland SWMF
01559> *#-----
01560>
01561> | ADD HYD (401 ) | ID: NHYD AREA QPEAK TPEAK R.V. DWF
01562> |-----|-----|-----|-----|-----|-----|
01563> | | | (ha) (cms) (hrs) (mm) (cms)
01564> | ID2 02:201b .20 .048 12.00 88.60 .000
01565> | +ID2 04:202b 5.80 1.353 12.00 88.60 .000
01566> |-----|-----|-----|-----|-----|-----|
01567> | SUM 05:401 6.00 1.401 12.00 88.60 .000
01568>
01569> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01570>
01571> 006:0008-----
01572> *#-----
01573> *# Constructed wetland SWMF
01574> *#-----
01575>
01576> | ROUTE RESERVOIR | Requested routing time step = 1.0 min.
01577> | IN>05: (401 ) |
01578> | OUT>06: (DP1 ) |
01579>
01580> ----- OUTFLOW STORAGE TABLE -----
01581> OUTFLOW STORAGE OUTFLOW STORAGE
01582> (cms) (ha.m.) | (cms) (ha.m.)
01583> .000 .0000E+00 | .121 .2361E+00
01584> .003 .2890E-01 | .136 .2763E+00
01585> .005 .5950E-01 | .150 .3180E+00
01586> .016 .3160E-01 | .459 .3613E+00
01587> .049 .1253E+00 | 1.087 .4063E+00
01588> .081 .1607E+00 | 2.002 .4528E+00
01589> .103 .1976E+00 | .000 .0000E+00
01590>
01591> ROUTING RESULTS AREA QPEAK TPEAK R.V.
01592> (ha) (cms) (hrs) (mm)
01593> INFLOW >05: (401 ) 6.00 1.401 12.00 88.598
01594> OUTFLOW<06: (DP1 ) 6.00 .147 12.917 88.595
01595>
01596> PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.504
01597> TIME SHIFT OF PEAK FLOW (min)= 55.00
01598> MAXIMUM STORAGE USED (ha.m.)=.3096E+00
01599>
01600> 006:0009-----
01601> *# Solar module area draining to diversion swale at west side of module
01602> *#-----
01603>
01604> | DESIGN NASHYD | Area (ha)= 1.70 Curve Number (CN)=79.00
01605> | ID7:202c DT= 5.00 | Ia (mm)= 1.500 # of Linear Res.(N)= 3.00
01606> | U.H. Tp(hrs)= .420
01607>
01608> Unit Hyd Qpeak (cms)= .155
01609>
01610> PEAK FLOW (cms)= .173 (i)
01611> TIME TO PEAK (hrs)= 12.250
01612> RUNOFF VOLUME (mm)= 67.394
01613> TOTAL RAINFALL (mm)= 110.602
01614> RUNOFF COEFFICIENT = .609
01615>
01616> (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
01617>
01618> 006:0010-----
01619> *#-----
01620>

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01621> | ADD HYD (woutSW) | ID: NHYD      AREA      QPEAK      TPEAK      R.V.      DWF
01622> -----|-----|-----|-----|-----|-----|-----|
01623>          ID1 01:201a      (ha)      (cms)      (hrs)      (mm)      (cms)
01624>          +ID2 02:201b      10.90      .753      12.67      68.96      .000
01625>          +ID3 03:202a      6.30      .424      12.67      67.39      .000
01626>          +ID4 04:202b      5.80      1.353      12.00      88.60      .000
01627>          +ID5 07:202c      1.70      .173      12.25      67.39      .000
01628> -----|-----|-----|-----|-----|-----|
01629>          SUM 08:woutSW      24.90      2.025      12.02      73.19      .000
01630>
01631> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01632>
01633> -----|-----|-----|-----|-----|-----|
01634> 006:0011-----|-----|-----|-----|-----|
01635> *#-----|-----|-----|-----|-----|
01636> -----|-----|-----|-----|-----|-----|
01637> | ADD HYD (withSW) | ID: NHYD      AREA      QPEAK      TPEAK      R.V.      DWF
01638> -----|-----|-----|-----|-----|-----|
01639>          ID1 01:201a      (ha)      (cms)      (hrs)      (mm)      (cms)
01640>          +ID2 03:202a      6.30      .424      12.67      67.39      .000
01641>          +ID3 06:DP1      6.00      .147      12.92      88.59      .000
01642>          +ID4 07:202c      1.70      .173      12.25      67.39      .000
01643> -----|-----|-----|-----|-----|-----|
01644>          SUM 10:withSW      24.90      1.456      12.58      73.19      .000
01645>
01646> NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
01647>
01648> -----|-----|-----|-----|-----|-----|
01649> 006:0012-----|-----|-----|-----|-----|
01650> *#-----|-----|-----|-----|-----|
01651> -----|-----|-----|-----|-----|-----|
01652> 006:0002-----|-----|-----|-----|-----|
01653> -----|-----|-----|-----|-----|-----|
01654> 006:0002-----|-----|-----|-----|-----|
01655> -----|-----|-----|-----|-----|-----|
01656> 006:0002-----|-----|-----|-----|-----|
01657> -----|-----|-----|-----|-----|-----|
01658> 006:0002-----|-----|-----|-----|-----|
01659> -----|-----|-----|-----|-----|-----|
01660> 006:0002-----|-----|-----|-----|-----|
01661> FINISH
01662> -----|-----|-----|-----|-----|-----|
01663> *****
01664> WARNINGS / ERRORS / NOTES
01665> -----|-----|-----|-----|-----|-----|
01666> Simulation ended on 2011-01-28 at 15:21:08
01667> -----|-----|-----|-----|-----|-----|
01668>

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Grand Renewable Energy Park - O & M Facility
Samsung Renewable Energy Inc.
O&M SWM Facility - Drainage Area Characteristics and Storage Requirements

Drainage Areas
(See below)

Total Area Tributary to Basin (ha)	6.00
Tributary Area requiring quality control (ha)	6.00
MOE Quality Control Requirement Basin Design	Enhanced Wetland
¹ Quality Control Volume Requirement (m ³ /ha)	99
² Permanent Pool (m ³)	353
³ Extended Detention - Quality Control (m ³)	240

¹ Based on MOE guidelines and overall percent impervious

²Permanent Pool sized for quality control - All but 40 m³/ha of required quality control volume

³Extended Detention sized for quality control - 40 m³/ha

Catchment Number	Area (ha)	% Imperv (XIMP)
201b	0.20	50%
201c	5.80	50%
Quality Control Area	6.00	50.0%
Quantity Control Area	6.00	50.0%

**Grand Renewable Energy Park
Samsung Renewable Energy Inc.
Operations and Maintenance Facility SWM Basin Stage-Storage-Discharge Calculations**

Rating Curve					
Elevation (m)	Discharge (m³/s)	Total SWMF Storage (m³)	Active Storage (m³)	Drawdown (hrs)	
				Increment	Total
195.80					
195.90		9			
196.00		22			
196.10		40			
196.20		63			
196.30		90			
196.40		122			
196.50		159			
196.60		200			
196.70		247			
196.80		297			
196.90		353			
197.00		413			
197.10		645			
197.20		896			
197.30		1,168			
197.40	0.003	1,457	289	27.4	27.4
197.50	0.005	1,763	595	22.1	49.5
197.60	0.016	2,084	916	8.6	58.0
197.70	0.049	2,421	1,253	2.9	60.9
197.80	0.081	2,775	1,607	1.5	62.4
197.90	0.103	3,144	1,976	1.1	63.6
198.00	0.121	3,529	2,361	1.0	64.5
198.10	0.136	3,931	2,763	0.9	65.4
198.20	0.150	4,348	3,180	0.8	66.2
198.30	0.454	4,781	3,613	0.4	66.6
198.40	1.062	5,231	4,063	0.2	66.8
198.50	1.932	5,696	4,528	0.1	66.8

Volume Estimation							
Elevation (m)	Forebay		Main Pond		Total Pond		Total SWMF
	Area (m²)	Volume (m³)	Area (m²)	Volume (m³)	Area (m²)	Volume (m³)	Volume (m³)
195.80	63						
195.90	110	9					9
196.00	157	22					22
196.10	204	40					40
196.20	250	63					63
196.30	297	90					90
196.40	344	122					122
196.50	391	159					159
196.60	438	200					200
196.70	485	247					247
196.80	532	297					297
196.90	579	353					353
197.00	625	413	1,594				413
197.10	672	478	1,745	167			645
197.20	719	547	1,896	349			896
197.30	766	622	2,047	546	2,813		1,168
197.40					2,973	289	1,457
197.50					3,133	595	1,763
197.60					3,293	916	2,084
197.70					3,453	1,253	2,421
197.80					3,613	1,607	2,775
197.90					3,774	1,976	3,144
198.00					3,934	2,361	3,529
198.10					4,094	2,763	3,931
198.20					4,254	3,180	4,348
198.30					4,414	3,613	4,781
198.40					4,574	4,063	5,231
198.50					4,734	4,528	5,696

Outlet Structure Controls						
Elevation (m)	Orifice 1 (m³/s)	Orifice 2 (m³/s)	Weir (m³/s)	Total Flow (m³/s)	Outlet Structure Characteristics	
					Outlet Structure Characteristics	
195.80					Orifice 1	
195.90					Orifice Invert Elev. (m)	Orifice Coeff.
196.00					197.30	0.60
196.10					Orifice Mid-point Elev. (m)	Perimeter (m)
196.20					197.34	0.24
196.30					Orifice Diam.(mm)	Area (m²)
196.40					75	0.004
196.50					Weir Coeff. (semi-circular)	Orientation
196.60					1.62	Vertical
196.70					Orifice 2	
196.80					Orifice Invert Elev. (m)	Orifice Coeff.
196.90					197.50	0.60
197.00					Orifice Mid-point Elev. (m)	Perimeter (m)
197.10					197.65	0.94
197.20					Orifice Diam.(mm)	Area (m²)
197.30					300	0.071
197.40	0.003			0.003	Weir Coeff. (semi-circular)	Orientation
197.50	0.005			0.005	1.62	Vertical
197.60	0.006	0.010		0.016	Emergency Overflow Weir	
197.70	0.007	0.042		0.049	Weir Invert	Weir Length
197.80	0.008	0.073		0.081	198.20	5
197.90	0.009	0.094		0.103	Weir Coeff. (rect.)	Weir Side Slopes (H:V) (? :1)
198.00	0.010	0.111		0.121	1.700	5
198.10	0.010	0.126		0.136	Weir Coeff. (tri.)	
198.20	0.011	0.139		0.150	0.600	
198.30	0.012	0.151	0.291	0.454		
198.40	0.012	0.163	0.887	1.062		
198.50	0.013	0.173	1.746	1.932		

Orifice Equation Used: Orifice flow equation

$$Q = C \cdot A \cdot (2 \cdot g \cdot h)^{0.5}$$

where

C = orifice coefficient

A = area of orifice

g = acceleration due to gravity

h = head above centre line of orifice

Note: Sharp crested weir equation with equivalent linear length used for calculating orifice flow rates when head is below centre line

Sharp crested semi-circular weir equation

$$Q = C \cdot D^{2.5} \cdot (H/D)^{1.88}$$

where

C = sharp crested semi-circular weir coefficient

D = diameter of orifice

H = head above orifice invert

Note: used when water elevation is below mid-point of orifice

Weir Equation Used: $Q = (C_{rectangle} \cdot L \cdot H^{3/2}) + ((C_{triangle} \cdot (8/15 \cdot (2 \cdot g)^{1/2} \cdot \tan(\Theta/2) \cdot H^{5/2}))$

where

L = bottom width of weir

H = head above weir invert

S = side slopes (ratio of H:V)

$C_{triangle}$ = triangular weir coefficient

$C_{rectangle}$ = broad-crested rectangular weir coefficient

g = 9.81 m/s²

$\Theta/2$ = angle formed by trapezoidal weir side slopes

**Grand Renewable Energy Park
Operations and Maintenance Facility SWM Basin
Sediment Forebay Sizing Calculations**

Using MOE - SWMPD Manual Criteria (2003)

STORMWATER MANAGEMENT FACILITY

Settling

$$\text{Dist} = \sqrt{r \cdot Q_p / v_s}$$

$$= 5.5 \quad \text{m}$$

r : 1 = l to w ratio
Q_p = peak SWM outflow for water quality portion of E.D. zone
v_s = settling velocity for 0.15 mm particles (m/s)

r = 3.00
Q_p = 0.0030
v_s = 0.0003

Dispersion Length (not applicable given the swale/ditch character of inlet conveyance - i.e. no jet dispersion)

$$\text{Dist} = 8Q/dv$$

$$= \text{n/a} \quad \text{m}$$

Q = 10 yr max inlet flow (m³/s)
d = depth of perm pool in forebay (m)
v_f = desired vel in forebay (m/s)

Q = n/a
d = 1
v_f = 0.5

Velocity

$$v = Q/A$$

$$= 0.04 \quad \text{m/s}$$

y = total depth of forebay from perm. pool (m)
b = bottom width (avg) of forebay (m)
Q = 10 yr inlet flow (m³/s)
A = cross-sectional area (m²)
Target velocity = 0.15

y = 1
b = 2
Q = 0.878
A = 24
V_{targ} = 0.15

Note 1.

Note 1.

Therefore, **Velocity Target Satisfied**

Cleanout Frequency

Table 6.3 MOE SWMPD Guidelines

$$\text{cleanout} = \text{Vol}/(\text{load} \cdot A_{\text{sew}} \cdot \text{effic})$$

$$= 11.9 \quad \text{years}$$

A_{sew} = Contributing Sewer Area (ha)
Imp = Percent Impervious (%)
load = Sediment Loading (m³/ha)
effic = Removal Efficiency (%)
Targ = Cleanout Frequency Target (years)
Vol = Sediment volume (m³) (0.5m depth)

A_{sew} = 6.00
Imp = 50%
load = 1.6
effic = 80%
Targ = 7
Vol = 90

Note 2.

Note 3.

Therefore, **Cleanout Time OK**

Surface Area Check

$$SA_f/SA_{pp} = 27.2\%$$

SA_f = Forebay Surface Area (m²)
SA_{pp} = Total Permanent Pool Surface Area (m²)
Targ = Forebay size (as % of Permanent Pool Area)

SA_f = 766
SA_{pp} = 2,813
Targ = 20%

The recommended design parameter limiting the forebay area to 20% of the total permanent pool surface area is a reflection of the fact that the volumetric sizing criteria for constructed wetland-type SWM facilities relies on the wetland vegetation component of the facility to perform the majority of the sediment removal functions, as opposed to a wet pond facility that relies on the dilution properties of the permanent pool. In this instance, it should be noted that the permanent pool volumetric sizing requirements, as defined by the MOE 2003 SWMPD Manual, are achieved within the wetland component *without accounting for storage volume provided within the forebay*. Therefore, it is suggested that that the facility, as designed, achieves the targets of the MOE Design Manual.

Notes

1. Total depth and cross-sectional area are 'worst-case' values, representative of conditions just prior to sediment clean-out
2. Interpolated based on percent impervious
3. Volume of bottom 0.5 m depth, the maximum sediment accumulation depth